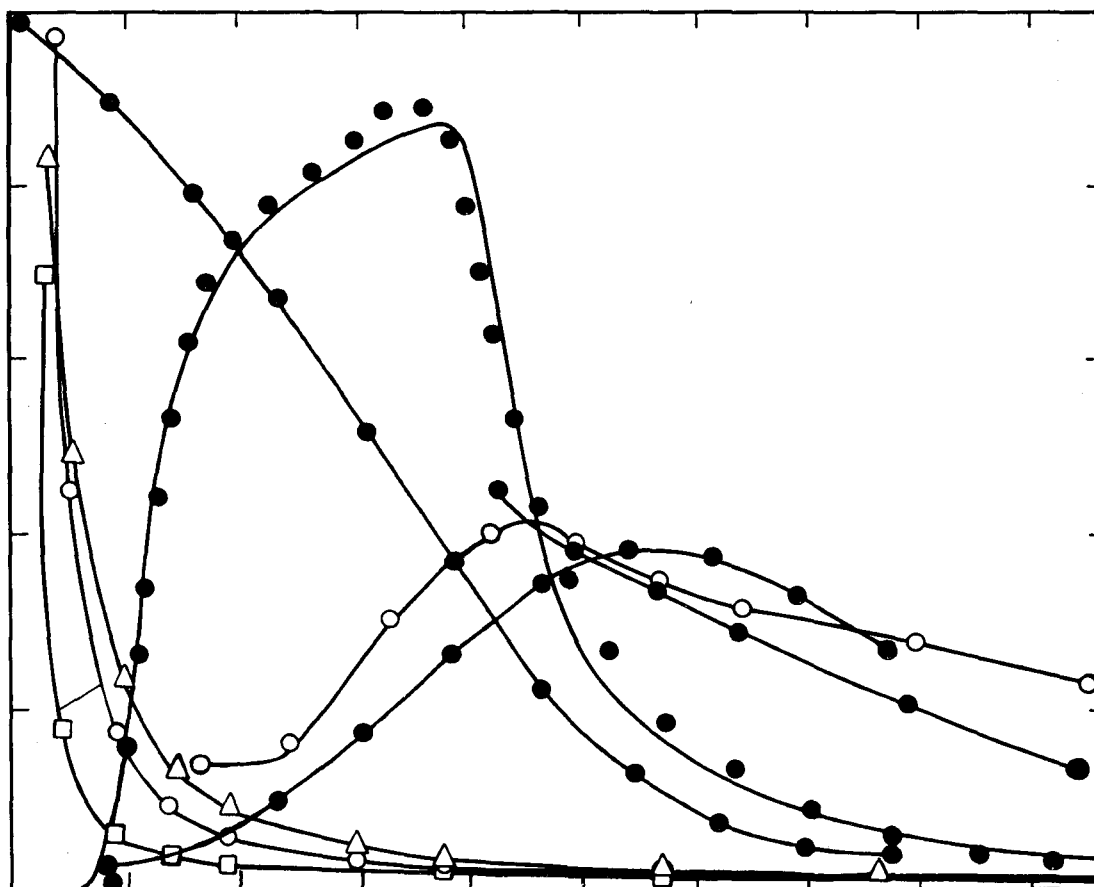


# Determining Transport Parameters from Laboratory and Field Tracer Experiments

J. C. Parker and M. Th. van Genuchten



DETERMINING TRANSPORT PARAMETERS FROM LABORATORY AND  
FIELD TRACER EXPERIMENTS

J. C. Parker  
Department of Agronomy  
Virginia Polytechnic Institute and State University  
Blacksburg, Virginia 24061

and

M. Th. van Genuchten  
U.S. Salinity Laboratory  
Riverside, California 92501

Virginia Agricultural Experiment Station  
Bulletin 84-3

May, 1984

**James R. Nichols, Dean and Director  
College of Agriculture and Life Sciences  
Virginia Agricultural Experiment Station  
Virginia Polytechnic Institute and State University  
Blacksburg, Virginia 24061**

The Virginia Agricultural and Mechanical College came into being in 1872 upon acceptance by the Commonwealth of the provisions of the Morrill Act of 1862 "to promote the liberal and practical education of the industrial classes in the several pursuits and professions of life." Research and investigations were first authorized at Virginia's land-grant college when the Virginia Agricultural Experiment Station was established by the Virginia General Assembly in 1886.

The Virginia Agricultural Experiment Station received its first allotment upon passage of the Hatch Act by the United States Congress in 1887. Other related Acts followed, and all were consolidated in 1955 under the Amended Hatch Act which states "It shall be the object and duty of the State agricultural experiment stations . . . to conduct original and other researches, investigations and experiments bearing directly on and contributing to the establishment and maintenance of a permanent and effective agricultural industry of the United States, including the researches basic to the problems of agriculture and its broadest aspects and such investigations as have for their purpose the development and improvement of the rural home and rural life and the maximum contributions by agriculture to the welfare of the consumer . . ."

In 1962, Congress passed the McIntire-Stennis Cooperative Forestry Research Act to encourage and assist the states in carrying on a program of forestry research, including reforestation, land management, watershed management, rangeland management, wildlife habitat improvement, outdoor recreation, harvesting and marketing of forest products, and "such other studies as may be necessary to obtain the fullest and most effective use of forest resources."

In 1966, the Virginia General Assembly "established within the Virginia Polytechnic Institute a division to be known as the Research Division . . . which shall encompass the now existing Virginia Agricultural Experiment Station . . ."

To simplify terminology, trade names of products or equipment may have been used in this publication, but no endorsement of products or firms mentioned is intended, nor is criticism implied of those not mentioned. Material appearing here may be reprinted provided no endorsement of a commercial product is stated or implied. Please credit the researchers involved and the Virginia Agricultural Experiment Station.

Virginia Tech does not discriminate against employees, students, or applicants on the basis of race, sex, handicap, age, veteran status, national origin, religion, or political affiliation. Anyone having questions concerning discrimination should contact the Equal Employment/Affirmative Action Office.





# ABSTRACT

J. C. Parker and M. Th. van Genuchten. 1984. Determining transport parameters from laboratory and field tracer experiments. Bulletin 84-3, Virginia Agricultural Experiment Station, Blacksburg.

This bulletin describes a nonlinear least-squares inversion method that can be used to identify several parameters in a number of theoretical one-dimensional solute transport models. One of the models discussed is the usual convection-dispersion transport equation that includes terms accounting for linear equilibrium adsorption, zero-order production and/or first-order decay. In addition, a two-site/two-region model is described that can be applied to various non-equilibrium transport problems. Also included is a stochastic model that considers the effects of areal variations in hydraulic fluxes on field-scale solute transport. This last model also has provisions for zero- or first-order production and/or decay. The least-squares inversion method can be used to analyze both spatial and temporal distributions of flux or resident concentrations. A detailed description of the computer program, called CXTFIT, is given in one of the appendices of this bulletin. Several example problems illustrating practical applications of the program are discussed in detail.

## CONTENTS

1.	INTRODUCTION. . . . .	1
2.	GENERAL FORMULATION OF THE TRANSPORT EQUATION . . . . .	2
3.	LINEAR EQUILIBRIUM ADSORPTION MODELS. . . . .	5
3.1.	Solution for $c_r$ with $\mu \neq 0$ . . . . .	6
3.2.	Solution for $c_r$ with $\mu = 0$ . . . . .	7
3.3.	Solution for $c_f$ with $\mu \neq 0$ . . . . .	8
3.4.	Solution for $c_f$ with $\mu = 0$ . . . . .	10
4.	TWO-SITE/TWO-REGION NON-EQUILIBRIUM MODELS. . . . .	11
4.1.	Solution for $c_r$ . . . . .	15
4.2.	Solution for $c_f$ . . . . .	17
5.	REGIONAL TRANSPORT MODELS . . . . .	17
5.1.	Solution for $\hat{c}_r$ . . . . .	19
5.2.	Solution for $\hat{c}_f$ . . . . .	22
6.	COMPUTER PROGRAM DESCRIPTION. . . . .	23
7.	APPLICATIONS. . . . .	31
8.	SUMMARY AND CONCLUSIONS . . . . .	46
9.	REFERENCES. . . . .	47
10.	APPENDIX A. List of the most significant variables in CXTFIT. . . . .	51
11.	APPENDIX B. Data input instructions. . . . .	54
12.	APPENDIX C. Input files for selected examples. . . . .	57
13.	APPENDIX D. Output files for selected examples . . . . .	63
14.	APPENDIX E. Fortran listing of CXTFIT. . . . .	81

LIST OF TABLES

Table 1. Characteristics of models designated in program CXTFIT. . . . .	27
Table 2. Comparison of fitted transport parameters for example 1 . . . . .	32
Table 3. Fitted parameter values and associated residual SSQ's for example 4 for two sets of initial estimates . . . . .	38
Table 4. Fitted model parameters for example 6 . . . . .	43



# LIST OF FIGURES

Figure 1. Hypothetical and fitted resident concentration distributions for deterministic linear equilibrium adsorption (example 1 using Model 1) . . . . .	32
Figure 2. Hypothetical and fitted flux concentration distributions for deterministic two-region/two-site nonequilibrium model (example 2 using Model 4) . . . . .	34
Figure 3. Experimental bromide effluent curve for packed sand with "wormhole", and fitted curves for bulk continuum model (Model 2) and two-region model (Model 4). . . . .	35
Figure 4. Experimental resident concentration distribution in "wormhole" column and predicted distribution from Model 1 using D fitted to the effluent data of Figure 3 with Model 2 . . . . .	36
Figure 5. Experimental and fitted boron effluent curves for Glendale clay loam (example 4 using Model 4) . . . . .	39
Figure 6. Hypothetical and fitted resident concentration distributions for stochastic transport model (example 5 using Model 7). . . . .	40

Figure 7. Experimental areally-averaged concentrations  $\bar{C}_r$  at three depths as a function of transformed time  $t^*$  for bromide transport in a field soil. (a) Data for 30 cm depth and fitted curve using Model 5; (b) data for 60 cm depth and predicted curve using parameter values fitted to the 30 cm depth data; (c) same as b for the 90 cm depth data . . . . . 44

## 1. INTRODUCTION

Concern about the behavior of various chemicals in the subsurface environment has resulted in the development of a number of theoretical models describing the basic processes of solute transport in soils. With the introduction of more and more sophisticated models, an important problem remaining is that of quantifying various model parameters, e.g., dispersion coefficients, retardation factors, and degradation constants. One popular method for determining these parameters is to fit them to observed laboratory or field displacement data. In particular, least-squares inversion methods have proved to be accurate and reliable tools for this identification process. Early examples using least-squares methods are given by Elprince and Day (1977), Laudelout and Dufey (1977), Agneessens et al. (1978) and Le Renard (1979).

In two previous reports we documented relatively simple computer programs that applied nonlinear least-squares inversion methods to deterministic equilibrium (van Genuchten, 1980) and various non-equilibrium adsorption models (van Genuchten, 1981). Those programs are applicable only to breakthrough curves in time, that is, to solute distributions determined at a fixed location downstream of an injection point. In addition, the previous programs ignored any production and decay processes and did not distinguish between analytical solutions applicable to different concentration detection modes. In this report we extend and modify the earlier programs to the analysis of resident- and flux-type concentration distributions that have been determined versus distance or versus time at more than one location in the soil profile. Zero- and first-order production or decay terms are included in the equilibrium adsorption model.

Spatial heterogeneity of medium properties often limits the applicability of deterministic approaches to mass transport for field-scale problems. In this report we also present a stochastic model that considers the effects of areal variations in hydraulic fluxes on transport for a solute subject to linear equilibrium adsorption and zero- and first-order production or decay. Calibration of this model using field observations of spatial and/or temporal resident or flux concentrations can be achieved by application of the least-squares inversion method.

After presenting the different analytical models, a description of the computer program is given, followed by a discussion of several examples that illustrate typical applications of the inversion method. Program listings, input instructions, and example input and output files are presented in various appendices. Machine-readable copies of the FORTRAN IV program are available from the authors upon request.

## 2. GENERAL FORMULATION OF THE TRANSPORT EQUATION

Transport of a single reactive solute species during steady fluid flow in a one-dimensional homogeneous system may be described by

$$\frac{\rho}{\theta} \frac{\partial s}{\partial t} + \frac{\partial c_r}{\partial t} = D \frac{\partial^2 c_r}{\partial x^2} - v \frac{\partial c_r}{\partial x} - \mu_w c_r - \frac{\mu_s \rho}{\theta} s + \gamma_w + \frac{\gamma_s \rho}{\theta} \quad [1]$$

where  $c_r$  is the volume-averaged resident concentration of the solute in the liquid phase ( $\text{ML}^{-3}$ ),  $s$  is the adsorbed concentration per unit mass of the solid phase ( $\text{MM}^{-1}$ ),  $x$  is distance (L),  $t$  is time (T),  $D$  is a dispersion coefficient reflecting the combined effects of diffusion and hydrodynamic dispersion on transport ( $\text{L}^2\text{T}^{-1}$ ),  $v$  is the average pore-water velocity ( $\text{LT}^{-1}$ ),  $\rho$  is the porous medium bulk density ( $\text{ML}^{-3}$ ) and  $\theta$  is the volumetric water content ( $\text{L}^3\text{L}^{-3}$ ). The coefficients  $\mu_w$  and  $\mu_s$  are rate constants for first-order decay in the liquid and solid phases of the soil, respectively ( $\text{T}^{-1}$ ). The coefficients  $\gamma_w$  and  $\gamma_s$  represent similar rate constants for zero-order production in the two soil phases ( $\text{ML}^{-3}\text{T}^{-1}$  and  $\text{T}^{-1}$ , respectively).

The solution of [1] requires an expression relating the adsorbed concentration ( $s$ ) to the solution concentration ( $c_r$ ). Various expressions for  $s$  have been presented in the literature for which analytical solutions of [1] can be obtained. This study considers deterministic solutions of [1] applicable to various linear equilibrium (section 3) and non-equilibrium (section 4) adsorption models. Also presented is a stochastic model that explicitly considers spatial heterogeneity in  $v$  and  $D$  for the case of linear equilibrium adsorption (section 5).

In addition to the adsorption term, Eq. [1] must be augmented with auxiliary equations describing the initial and boundary conditions of the system under study. In all cases, the initial condition is assumed to be of the simple form

$$c_r(x,0) = C_i \quad [2]$$

where  $C_i$  is a constant.

There has been much discussion in the literature about the type of boundary conditions that are most appropriate for finite and semi-infinite systems. In earlier studies (van Genuchten and Parker, 1984; Parker and van Genuchten, 1984), we showed that a third- or flux-type boundary condition should be used at the inlet position ( $x=0$ ), i.e.,

$$\left( c_r - \frac{D}{v} \frac{\partial c_r}{\partial x} \right) \bigg|_{x=0} = C_{in}(t) \quad [3]$$

where  $C_{in}(t)$  is the concentration of the injection fluid as a function of time. Equation [3] implies a discontinuity in concentration across the injection boundary, which increases with the value of the apparent dispersivity  $D/v$ . This discontinuity is a direct consequence of the assumption that at the injection plane a boundary layer of infinitesimal thickness develops in which the system parameters change discontinuously from those of a perfectly mixed inlet reservoir ( $x < 0$ ) to those of the bulk porous medium ( $x > 0$ ). Microscopically, this change always takes place over a finite transition region.

The lower boundary condition for an effectively semi-infinite system can be written as

$$\frac{\partial c_r}{\partial x}(\infty, t) = \text{finite} \quad [4]$$

For a finite system of length  $L$ , a frequently used boundary condition is

$$\frac{\partial c_r}{\partial x}(L, t) = 0 \quad [5]$$

This condition disregards the development of a boundary transition layer similar to that noted for the injection boundary. In effect, the concentration is forced to be continuous across the exit boundary. Accommodating the presence of a boundary layer at  $x = L$  leads to a discontinuous concentration distribution across the lower boundary, and thus to a gradient in  $c_r$  interior to the transition zone that is not constrained to be zero. So long as backmixing at the exit boundary is negligible (which generally is the case), Eq. [4] for the semi-infinite case can be used with impunity and the resulting solution for  $c_r(x, t)$  applied to the finite region  $0 < x < L$  as well (Parker and van Genuchten, 1984; Parker, 1984). Accordingly, all solutions in this report are based on [4].

In many instances, the experimental conditions are such that measured concentrations are flux-averaged rather than volume-averaged. This is the case when effluent concentrations from column tracer experiments, pan lysimeters or subphreatic wells are analyzed. To meaningfully interpret such measurements, it is imperative that the concentrations not be assumed to represent resident concentrations ( $c_r$ ) at the outflow boundary. Instead, they must be treated as representing flux-averaged concentrations ( $c_f$ ), which are defined by (Brigham, 1974; Kreft and Zuber, 1978):

$$c_f = J/q \quad [6]$$

where  $J$  and  $q$  are the solute and liquid flux densities, respectively:

$$J = qc_r - \theta D \frac{\partial c_r}{\partial x} \quad [7]$$

and

$$q = \theta v \quad [8]$$

With [7] and [8], Eq. [6] becomes

$$c_f = c_r - \frac{D}{v} \frac{\partial c_r}{\partial x} \quad [9]$$

Hence, the flux-averaged concentration,  $c_f$ , represents the "mass of solute per unit volume of fluid passing through a given cross-section during an elementary time interval" (Kreft and Zuber, 1978). In this report, we will use analytical solutions of [1] for both the volume-averaged concentration,  $c_r$ , and the flux-averaged concentration,  $c_f$ .

### 3. LINEAR EQUILIBRIUM ADSORPTION MODELS

Consider the case where adsorption is described by a linear or linearized isotherm of the form

$$s = kc_r \quad [10]$$

where  $k$  is an empirical distribution constant [ $L^3M^{-1}$ ]. Substituting [10] in [1] yields

$$R \frac{\partial c_r}{\partial t} = D \frac{\partial^2 c_r}{\partial x^2} - v \frac{\partial c_r}{\partial x} - \mu c_r + \gamma \quad [11]$$

where the dimensionless retardation factor  $R$  is defined as

$$R = 1 + \rho k / \theta \quad [12]$$

and the new rate coefficients  $\mu$  and  $\gamma$  ( $T^{-1}$  and  $ML^{-3}T^{-1}$ , respectively) are given by

$$\mu = \mu_w + \mu_s \rho k / \theta \quad [13]$$

$$\gamma = \gamma_w + \gamma_s \rho / \theta \quad [14]$$

We will consider solutions of [11] for pulse-type input boundary conditions of the form

$$\left( c_r - \frac{D}{v} \frac{\partial c_r}{\partial x} \right) \Big|_{x=0} = \begin{cases} C_o & 0 < t \leq t_o \\ 0 & t > t_o \end{cases} \quad [15]$$

where  $C_o$  is a constant. Because solutions for zero values of  $\mu$  do not follow directly from the more general solutions for non-zero  $\mu$ , the two cases of zero and non-zero first-order degradation will be considered separately. On the other hand, all solutions for  $\gamma = 0$  can be obtained immediately from those for non-zero  $\gamma$  by simply forcing this coefficient to be zero in the analytical solutions.

### 3.1. Solution for $c_r$ with $\mu \neq 0$ .

The analytical solution for this problem is (see case C6 of van Genuchten and Alves, 1982)

$$c_r(x, t) = \begin{cases} \left( \frac{Y}{\mu} + (C_i - \frac{Y}{\mu}) A(x, t) + (C_o - \frac{Y}{\mu}) B(x, t) \right) & 0 < t \leq t_o \\ \left( \frac{Y}{\mu} + (C_i - \frac{Y}{\mu}) A(x, t) + (C_o - \frac{Y}{\mu}) B(x, t) - C_o B(x, t - t_o) \right) & t > t_o \end{cases}$$

where

$$A(x, t) = \exp\left(-\frac{\mu t}{R}\right) \left\{ 1 - \frac{1}{2} \operatorname{erfc}\left[\frac{Rx - vt}{2(DRt)^{1/2}}\right] - \left(\frac{v^2 t}{\pi DR}\right)^{1/2} \exp\left[-\frac{(Rx - vt)^2}{4DRt}\right] \right\}$$



$$\begin{aligned}
& + \frac{1}{2} \left( 1 + \frac{vx}{D} + \frac{v^2 t}{DR} \right) \exp\left(\frac{vx}{D}\right) \operatorname{erfc}\left[\frac{Rx + vt}{2(DRt)^{1/2}}\right] \} \\
B(x,t) = & \frac{v}{v+u} \exp\left[\frac{(v-u)x}{2D}\right] \operatorname{erfc}\left[\frac{Rx - ut}{2(DRt)^{1/2}}\right] \\
& + \frac{v}{v-u} \exp\left[\frac{(v+u)x}{2D}\right] \operatorname{erfc}\left[\frac{Rx + ut}{2(DRt)^{1/2}}\right] \\
& + \frac{v^2}{2\mu D} \exp\left(\frac{vx}{D} - \frac{\mu t}{R}\right) \operatorname{erfc}\left[\frac{Rx + vt}{2(DRt)^{1/2}}\right]
\end{aligned}$$

and

$$u = (v^2 + 4\mu D)^{1/2}.$$

### 3.2. Solution for $c_r$ with $\mu = 0$ .

Because of a division by zero, the analytical solution above cannot be used for the special case when first-order decay is negligible. For  $\mu = 0$ , the solution reduces to (case B6 of van Genuchten and Alves, 1982)

$$c(x,t) = \begin{cases} C_1 + (C_0 - C_1) A(x,t) + B(x,t) & 0 < t \leq t_0 \\ C_1 + (C_0 - C_1) A(x,t) + B(x,t) - C_0 A(x, t-t_0) & t > t_0 \end{cases}$$

where

$$\begin{aligned}
A(x,t) = & \frac{1}{2} \operatorname{erfc}\left[\frac{Rx - vt}{2(DRt)^{1/2}}\right] + \left(\frac{v^2 t}{\pi DR}\right)^{1/2} \exp\left[-\frac{(Rx - vt)^2}{4DRt}\right] \\
& - \frac{1}{2} \left( 1 + \frac{vx}{D} + \frac{v^2 t}{DR} \right) \exp\left(\frac{vx}{D}\right) \operatorname{erfc}\left[\frac{Rx + vt}{2(DRt)^{1/2}}\right]
\end{aligned}$$

$$\begin{aligned}
B(x,t) = & \frac{\gamma}{R} \left\{ t + \left( \frac{Rx}{2v} - \frac{t}{2} + \frac{DR}{2v^2} \right) \operatorname{erfc} \left[ \frac{Rx - vt}{2(DRt)^{1/2}} \right] \right. \\
& - \left( \frac{t}{4\pi DR} \right)^{1/2} \left( Rx + vt + \frac{2DR}{v} \right) \exp \left[ - \frac{(Rx - vt)^2}{4DRt} \right] \\
& \left. + \left[ \frac{t}{2} - \frac{DR}{2v^2} + \frac{(Rx + vt)^2}{4DR} \right] \exp \left( \frac{vx}{D} \right) \operatorname{erfc} \left[ \frac{Rx + vt}{2(DRt)^{1/2}} \right] \right\}.
\end{aligned}$$

### 3.3. Solution for $c_f$ with $\mu \neq 0$ .

The solutions for the flux-averaged concentrations ( $c_f$ ) follow immediately from those of the volume-averaged concentrations ( $c_r$ ) above by making use of definition [9]. Alternatively, the desired expressions for  $c_f$  can be found by first using [9] to redefine transport equation [11] and its initial and boundary conditions in terms of the flux-averaged concentration, and subsequently solving the transformed set of equations. To accomplish the latter, let us first differentiate [9] with respect to  $x$  and  $t$ , to give respectively

$$\frac{\partial c_f}{\partial x} = \frac{\partial c_r}{\partial x} - \frac{D}{v} \frac{\partial^2 c_r}{\partial x^2} \quad [16]$$

$$\frac{\partial c_f}{\partial t} = \frac{\partial c_r}{\partial t} - \frac{D}{v} \frac{\partial^2 c_r}{\partial t \partial x} \quad [17]$$

Combining [11] and [16] gives

$$R \frac{\partial c_r}{\partial t} = -v \frac{\partial c_f}{\partial x} - \mu c_r + \gamma \quad [18]$$

Next we use [17] to eliminate  $\partial c_r / \partial t$  from [18]:

$$R \frac{\partial c_f}{\partial t} = -\frac{RD}{v} \frac{\partial^2 c_r}{\partial t \partial x} - v \frac{\partial c_f}{\partial x} - \mu c_r + \gamma$$

$$= -\frac{D}{v} \frac{\partial}{\partial x} \left( R \frac{\partial c_r}{\partial t} \right) - v \frac{\partial c_f}{\partial x} - \mu c_r + \gamma \quad [19]$$

which, again with [18] and some rearranging, yields

$$R \frac{\partial c_f}{\partial t} = D \frac{\partial^2 c_f}{\partial x^2} - v \frac{\partial c_f}{\partial x} - \mu \left( c_r - \frac{D}{v} \frac{\partial c_r}{\partial x} \right) + \gamma. \quad [20]$$

Finally, substitution of [9] into [20] leads to

$$R \frac{\partial c_f}{\partial t} = D \frac{\partial^2 c_f}{\partial x^2} - v \frac{\partial c_f}{\partial x} - \mu c_f + \gamma \quad [21]$$

which is identical to [11], except that the volume-averaged concentration,  $c_f$ , is replaced by the flux-averaged concentration,  $c_f$ .

The initial and boundary conditions are transformed in the same manner to give

$$c_f(x, 0) = c_i \quad [22]$$

$$\frac{\partial c_f}{\partial x}(\infty, t) = \text{finite} \quad [23]$$

$$c_f(0, t) = \begin{cases} c_o & 0 < t \leq t_o \\ 0 & t > t_o \end{cases} \quad [24]$$

The transformation from [4] to [23] formally requires that the second spatial derivative of  $c_r$  also be finite when  $x \rightarrow \infty$ . Using the different solutions for  $c_r$ , one may verify that this criterion is indeed always met. Note that the transport model for  $c_f$  is exactly the same as the model for  $c_r$ , except that the third-type input boundary condition for  $c_r$  has been transformed into a first-type condition for  $c_f$ .

The complete solution of Eqs. [21] through [24] is (case C5 of van Genuchten and Alves, 1982)

$$c_f(x,t) = \begin{cases} \frac{Y}{\mu} + (C_1 - \frac{Y}{\mu}) A(x,t) + (C_0 - \frac{Y}{\mu}) B(x,t) & 0 < t < t_0 \\ \frac{Y}{\mu} + (C_1 - \frac{Y}{\mu}) A(x,t) + (C_0 - \frac{Y}{\mu}) B(x,t) - C_0 B(x,t-t_0) & t > t_0 \end{cases}$$

where

$$A(x,t) = \exp(-\frac{\mu t}{R}) \left\{ 1 - \frac{1}{2} \operatorname{erfc}\left[\frac{Rx - vt}{2(DRt)^{1/2}}\right] - \frac{1}{2} \exp(\frac{vx}{D}) \operatorname{erfc}\left[\frac{Rx + vt}{2(DRt)^{1/2}}\right] \right\}$$

$$B(x,t) = \frac{1}{2} \exp\left[\frac{(v-u)x}{2D}\right] \operatorname{erfc}\left[\frac{Rx - ut}{2(DRt)^{1/2}}\right] + \frac{1}{2} \exp\left[\frac{(v+u)x}{2D}\right] \operatorname{erfc}\left[\frac{Rx + ut}{2(DRt)^{1/2}}\right]$$

and

$$u = (v^2 + 4\mu D)^{1/2}$$

### 3.4 Solution for $c_f$ with $\mu = 0$ .

The solution is (case B5 of van Genuchten and Alves, 1982)

$$c(x,t) = \begin{cases} C_1 + (C_0 - C_1) A(x,t) + B(x,t) & 0 < t < t_0 \\ C_1 + (C_0 - C_1) A(x,t) + B(x,t) - C_0 A(x,t-t_0) & t > t_0 \end{cases}$$

where

$$A(x,t) = \frac{1}{2} \operatorname{erfc}\left[\frac{Rx - vt}{2(DRt)^{1/2}}\right] + \frac{1}{2} \exp(\frac{vx}{D}) \operatorname{erfc}\left[\frac{Rx + vt}{2(DRt)^{1/2}}\right]$$

$$B(x,t) = \frac{Y}{R} \left\{ t + \frac{(Rx-vt)}{2v} \operatorname{erfc}\left[\frac{Rx-vt}{2(DRt)^{1/2}}\right] - \frac{(Rx+vt)}{2v} \exp\left(\frac{vx}{D}\right) \operatorname{erfc}\left[\frac{Rx+vt}{2(DRt)^{1/2}}\right] \right\}.$$

#### 4. TWO-SITE/TWO-REGION NON-EQUILIBRIUM MODELS

We now consider the case where the adsorption term in [1] consists of two components, one governed by equilibrium adsorption and one by first-order kinetic non-equilibrium adsorption. This type of "two-site" adsorption model has been discussed by Selim et al. (1976), Cameron and Klute (1977), Rao et al. (1979), De Camargo et al. (1979), Hoffman and Rolston (1980) and by Fluhler and Jury (1983). Basic to the two-site adsorption model is the idea that the solid phase of the soil is made up of different constituents (soil minerals, organic matter, iron and aluminum oxides), and that a chemical is likely to react with these different constituents at different rates and with different intensities. The model assumes that sorption sites can be divided into two fractions; adsorption on one fraction ("type-1" sites) is assumed to be instantaneous, while adsorption on the other fraction ("type-2" sites) is thought to be time-dependent. As was the case in the earlier report (van Genuchten, 1981), we will ignore any production or decay processes for the non-equilibrium models.

At equilibrium, adsorption on both the equilibrium and kinetic sites is described by linear equations:

$$\begin{aligned} s_1 &= k_1 c_r \\ &= Fk c_r \end{aligned} \quad [25]$$

$$\begin{aligned}
 s_2 &= k_2 c_r \\
 &= (1-F)kc_r
 \end{aligned}
 \tag{26}$$

where the subscripts 1 and 2 refer to type-1 (equilibrium) and type-2 (kinetic) sites, respectively, and where F is the fraction of all sites occupied by type-1 sorption sites. Total adsorption at equilibrium is simply

$$s = s_1 + s_2 = kc_r \tag{27}$$

Because type-1 sites are always at equilibrium, it follows from [25] that

$$\frac{\partial s_1}{\partial t} = Fk \frac{\partial c_r}{\partial t} \tag{28}$$

The adsorption rate for the type-2 kinetic non-equilibrium sites is given by a linear and reversible rate equation of the form

$$\frac{\partial s_2}{\partial t} = \alpha(k_2 c_r - s_2) \tag{29}$$

where  $\alpha$  is a first-order rate coefficient ( $T^{-1}$ ). Ignoring the production and decay terms in Eq. (1) and combining this equation with the adsorption expressions above leads to the following transport model:

$$\left(1 + \frac{F\rho k}{\theta}\right) \frac{\partial c_r}{\partial t} + \frac{\rho}{\theta} \frac{\partial s_2}{\partial t} = D \frac{\partial^2 c_r}{\partial x^2} - v \frac{\partial c_r}{\partial x} \tag{30}$$

$$\frac{\partial s_2}{\partial t} = \alpha[(1-F)kc_r - s_2] \tag{31}$$

As in previous cases, Eqs. [30] and [31] will be solved for a uniform initial concentration (Eq. 2) and for a pulse-type injection (Eq. 15) into a semi-infinite medium. For  $s_2$  an additional initial condition is required:

$$s_2(x,0) = (1-F)kC_1. \quad [32]$$

The following dimensionless variables are introduced:

$$T = vt/L \quad [33]$$

$$z = x/L \quad [34]$$

$$P = vL/D \quad [35]$$

$$R = 1 + \rho k/\theta \quad [36]$$

$$\beta = \frac{\theta + F\rho k}{\theta + \rho k} \quad [37]$$

$$\omega = \alpha(1-\beta)RL/v \quad [38]$$

$$c_1 = \frac{c_r - C_1}{C_o - C_1} \quad [39]$$

$$c_2 = \frac{s_2 - (1-F)kC_1}{(1-F)k(C_o - C_1)} \quad [40]$$

where  $L$  is an arbitrary positive distance from the origin. Substituting these variables into [30] and [31] yields

$$\beta R \frac{\partial c_1}{\partial T} + (1-\beta)R \frac{\partial c_2}{\partial T} = \frac{1}{P} \frac{\partial^2 c_1}{\partial z^2} - \frac{\partial c_1}{\partial z} \quad [41]$$

$$(1-\beta)R \frac{\partial c_2}{\partial T} = \omega(c_1 - c_2). \quad [42]$$

As pointed out by van Genuchten (1981), a mathematical problem identical to that of the two-site kinetic model is obtained when apparent non-equilibrium conditions in the system are attributed to large heterogeneities in microscopic pore-water velocities. This approach assumes that the liquid phase can be partitioned into "mobile" (dynamic or macro-porosity) and "immobile" (stagnant or micro-porosity) regions. Convective and dispersive transport is restricted to the mobile water phase, while transfer of solutes into and out of the immobile (nonmoving) liquid phase is assumed to be diffusion-limited. The governing equations for this two-region model are

$$(\theta_m + f\rho k) \frac{\partial c_m}{\partial t} + [\theta_{im} + (1-f)\rho k] \frac{\partial c_{im}}{\partial t} = \theta_m D_m \frac{\partial^2 c_m}{\partial x^2} - q \frac{\partial c_m}{\partial x} \quad [43]$$

$$[\theta_{im} + (1-f)\rho k] \frac{\partial c_{im}}{\partial t} = \alpha^* (c_m - c_{im}) \quad [44]$$

where  $c_m$  and  $c_{im}$  are the resident concentrations of the mobile and immobile liquid phases, respectively;  $\theta_m$  and  $\theta_{im}$  are the mobile and immobile volumetric water contents such that  $\theta = \theta_m + \theta_{im}$ ,  $D_m$  is the dispersion coefficient for the mobile region,  $f$  represents the fraction of the sorption sites that equilibrates with the mobile liquid phase, and  $\alpha^*$  is a first-order rate constant that governs the rate of solute exchange between the mobile and immobile regions.

The dimensionless form of the two-site model is identically preserved when [43] and [44] are expressed in terms of  $T$ ,  $z$  and  $R$  (Eqs. [33], [34] and [36]), and the following reduced variables:

$$P = v_m L / D_m \quad [45]$$



$$\beta = \frac{\theta_m + f\rho k}{\theta + \rho k} \quad [46]$$

$$\omega = \alpha^* L/q \quad [47]$$

$$c_1 = \frac{c_m - c_i}{c_o - c_i} \quad [48]$$

$$c_2 = \frac{c_{im} - c_i}{c_o - c_i} \quad [49]$$

where  $v_m = q/\theta_m$ . Note that [45] is the same as [35] if we define  $D$  for the two-region model as  $D = D_m \theta_m / \theta$ . Because the dimensionless transport equations and their initial and boundary conditions are identical, parameters obtained by fitting data to the two-site adsorption model may be interpreted also in terms of a two-region physical non-equilibrium model (and vice-versa) if deemed appropriate.

#### 4.1. Solution for $c_r$ .

Analytical solutions of Eqs. [41] and [42], or of mathematically similar equations, have been derived for a variety of initial and boundary conditions (Lapidus and Amundson, 1952; Coats and Smith, 1964; Villiermaux and van Swaay, 1969; Bennet and Goodridge, 1970; Lindstrom and Narasimhan, 1973; Lindstrom and Stone, 1974; van Genuchten, 1974; Lindstrom and Boersma, 1975; Lindstrom, 1976; Cameron and Klute, 1977; Popovic and Deckwer, 1976). As shown by De Smedt and Wierenga (1979), these solutions can all be expressed in the same general format. For the initial and boundary conditions of this study, the solution for the volume-averaged concentration ( $c_r$ ) in terms of the reduced variables given in the previous section is (see also van Genuchten, 1981):

$$c_r(z, T) = \begin{cases} C_i + (C_o - C_i) A(z, T) & 0 < T \leq T_o \\ C_i + (C_o - C_i) A(z, T) - C_o A(z, T - T_o) & T > T_o \end{cases} \quad [50]$$

where

$$A(z, T) = \int_0^T g(z, \tau) J(a, b) d\tau \quad [51]$$

$$g(z, \tau) = \left(\frac{P}{\pi \beta R \tau}\right)^{1/2} \exp\left[-\frac{P(\beta R z - \tau)^2}{4 \beta R \tau}\right] - \frac{P}{2 \beta R} \exp(Pz) \operatorname{erfc}\left[\left(\frac{P}{4 \beta R \tau}\right)^{1/2} (\beta R z + \tau)\right] \quad [52]$$

$$J(a, b) = 1 - e^{-b} \int_0^a e^{-\lambda} I_0[2\sqrt{b\lambda}] d\lambda \quad [53]$$

$$a = \frac{\omega \tau}{\beta R} \quad [54]$$

and

$$b = \frac{\omega(T - \tau)}{(1 - \beta)R}. \quad [55]$$

The function  $J(a, b)$  above is often referred to as Goldstein's J-function (Goldstein, 1953);  $I_0$  in this function represents a zero-order Bessel function. Some properties and computational approximations of the J-function are summarized elsewhere (van Genuchten, 1981). We emphasize here that the analytical solution for  $c_r$  above represents the volume-average concentration of the entire liquid phase if applied to the two-site model, while for the two-region model the solution represents the volume-averaged concentration of the "mobile" liquid phase only.

#### 4.2. Solution for $c_f$ .

The analytical solution for the flux-averaged concentration ( $c_f$ ) follows immediately if one applies transformation [9] to the analytical solution for  $c_r$  given in the previous section. The solution is exactly the same as before, except that [52] is replaced by

$$g(z, \tau) = \frac{z}{\tau} \left( \frac{P\beta R}{4\pi\tau} \right)^{1/2} \exp\left[-\frac{P(\beta Rz - \tau)^2}{4\beta R\tau}\right]. \quad [56]$$

One may verify that the solution for  $c_f$  thus obtained is the analytical solution of Eqs. [41] and [42] for a semi-infinite medium subject to a first-type input boundary condition of the same form as given by Eq. [24] of section 3.3.

#### 5. REGIONAL TRANSPORT MODELS

Growing evidence in the literature indicates that deterministic solutions of the convection-dispersion equation may not adequately describe solute transport in natural porous media at the field-scale (Gelhar et al., 1979; Bresler and Dagan, 1981; Pickens and Grisak, 1981; Amoozegar-Fard et al., 1982; Simmons, 1982; Tang et al., 1982). At least in part, this inadequacy is caused by medium heterogeneities which often increase significantly with the scale of observation. The two-region model discussed in the preceding section represents an attempt to accommodate pore structure variability at a scale intermediate between the usual laboratory measurements and the larger field scale. To evaluate the effects of field-scale heterogeneities, the stochastic nature of the transport process must be explicitly dealt with. The model formulated below is similar to the one-dimensional stochastic transport models of Bresler and Dagan (1981) and Amoozegar-Fard et al. (1982). Conceptually, we regard the transport region as being composed of numerous parallel porous columns (which we denote as the "local scale"), each having specific properties and being subject to

specific local boundary conditions. We further assume that transport within each column can be described by the one-dimensional convection-dispersion equation (Eq. 11) with deterministic coefficients. Lateral flow, transverse dispersion, and vertical inhomogeneities are thus regarded to be negligible.

Variations in local pore water velocities are considered to be log-normally distributed. Because the flow region is assumed implicitly to be fixed, a unique realization of the stochastic variables will be obtained and the actual spatial pattern of the velocity distribution is of no concern. The probability density function for an idealized log-normal distribution of  $v$  is

$$\rho(v) = \frac{1}{v\sigma_{1n}(2\pi)^{1/2}} \exp\left\{-\frac{[\ln(v) - \mu_{1n}]^2}{2\sigma_{1n}^2}\right\} \quad [57]$$

where  $\mu_{1n}$  and  $\sigma_{1n}$  are the mean and standard deviation of  $\ln(v)$ . The density function is normalized so as to yield unity for the integral of  $\rho(v)$  from  $v = 0$  to  $\infty$ . The first moment,  $\langle v \rangle$ , of this density function gives the expected value of  $v$ :

$$\langle v \rangle = \frac{\int_0^\infty v \rho(v) dv}{\int_0^\infty \rho(v) dv} \quad [58]$$

which yields

$$\langle v \rangle = \exp\left(\mu_{1n} + \frac{1}{2} \sigma_{1n}^2\right). \quad [59]$$

Hence,  $\langle v \rangle$  represents the mean field value of  $v$ .

Dispersion coefficients measured at the scale of single solution sampling devices have also been found to vary log-normally (Biggar and Nielsen, 1976). These variations, however, are not independent of the observed pore water velocity variations and may, to a first approximation, be described by the relation

$$D = \epsilon v$$

[60]

where  $\epsilon$  is the local-scale dispersivity (L). We assume  $\epsilon$  to be deterministic and constant within the flow region. This assumption effectively means that  $\rho(D)$  is defined completely in terms of  $\rho(v)$  and  $\epsilon$ . Sensitivity analyses suggest that variations in  $D$  beyond those linked directly to  $v$  through [60] are insignificant compared to the effects of areal velocity variations on local convective transport (Amoozegar-Fard et al., 1982).

We further assume that water contents, adsorption coefficients and the zero- and first-order reaction constants are all deterministic variables. Thus the medium properties are completely defined by the deterministic coefficients  $R$ ,  $\mu$ ,  $\gamma$  and  $\epsilon$  and the stochastic probability density function  $\rho(v)$ , characterized by the coefficients  $\mu_{1n}$  and  $\sigma_{1n}$ . We now consider solutions for field-scale resident and flux concentrations pertinent to specified field-scale initial and boundary conditions.

#### 5.1. Solution for $\hat{c}_r$ .

The field-scale resident concentration  $\hat{c}_r$  represents the mean concentration value occurring over any plane perpendicular to the velocity field and parallel to the injection boundary:

$$\hat{c}_r = \frac{\int_{(A)} c_r dA}{\int_{(A)} dA} \quad [61]$$

where  $c_r$  is the local-scale concentration and  $A$  is the areal domain of interest. Since velocity is the only stochastic variable over the areal domain, [61] may be written as

$$\hat{c}_r = \frac{\int_0^\infty c_r \rho(v) dv}{\int_0^\infty \rho(v) dv} \quad [62]$$

where  $\hat{c}_r = \hat{c}_r(x, t)$  and  $c_r = c_r(x, t, v)$ . By definition of the normalized density function, the denominator in [62] is unity. Equation

[62] indicates that  $\hat{c}_r$  represents the expected value of the random variable  $c_r$  (denoted as  $\langle c_r \rangle$ ).

To solve [62], we must evaluate the local value of  $c_r$ , which is assumed to be described by analytical solutions of section 3.1 and 3.2 for a uniform initial condition and a pulse-type injection at the local scale. The initial and input concentrations ( $C_i$  and  $C_o$ ) in these solutions are assumed to be deterministic. The field-scale injection boundary condition can be formulated in one of two ways. To stipulate these conditions, let us first introduce the cumulative local inlet solute flux  $M$  ( $ML^{-2}$ ):

$$M(t) = \int_0^t J_o(\tau) d\tau \quad [63]$$

where  $J_o$  is the local mass flux density at the inlet boundary. For a pulse-type injection, [63] yields

$$M(t) = \begin{cases} v\theta C_o t & 0 < t < t_o \\ v\theta C_o t_o & t > t_o \end{cases} \quad [64]$$

We also define  $M_o \equiv M(t \rightarrow \infty) = v\theta C_o t_o$  which represents the total amount of material to be added to the profile. The two field-scale boundary conditions for which we will obtain solutions are determined by the following situations:

- (1) Deterministic  $t_o$ ,  $\theta$ ,  $C_o$ ; Stochastic  $M_o$ ,  $v$ .
- (2) Deterministic  $M_o$ ,  $\theta$ ,  $C_o$ ; Stochastic  $t_o$ ,  $v$ .

Thus either the pulse duration ( $t_o$ ) or the asymptotic value of the cumulative influx ( $M_o$ ) may be taken as constant.

By fixing  $t_o$ , it is implied that  $M$  and  $M_o$  vary locally in direct proportion to  $v$ . The mean value  $\langle M \rangle$  of  $M$  at the field scale is then

$$\langle M \rangle(t) = \begin{cases} \langle v \rangle \theta C_0 t & 0 < t < t_0 \\ \langle v \rangle \theta C_0 t_0 & t > t_0 \end{cases} \quad [65]$$

As before, let  $\langle M_0 \rangle \equiv \langle v \rangle \theta C_0 t_0$ . Equation [65] should be used when a tracer solution is applied uniformly over the flow region for a fixed time period.

A second field-scale boundary condition can be formulated by stipulating a uniform local cumulative flux  $M_0 = M(t \rightarrow \infty)$ , thus indicating that  $t_0$  varies inversely with  $v$  in accordance with [64]. This condition is appropriate when chemicals are added uniformly over an area in solid form and subsequently leached continuously with a solute-free solution. In that case,  $C_0$  may be viewed as being governed by the chemical solubility. The temporally integrated areal mean mass flux is now given by

$$\langle M \rangle(t) = \int_0^t \int_0^{v_0} \theta C_0 \rho(v) dv d\tau \quad (v_0 = \frac{M_0}{\tau \theta C_0}) \quad [66]$$

which in the limit when  $t \rightarrow \infty$  yields  $\langle M \rangle(\infty) = \langle M_0 \rangle = M_0$ .

The initial and boundary conditions for the regional transport models are completely defined by the constants  $C_1$ ,  $C_0$  and  $t_0$  or by  $C_1$ ,  $C_0$ ,  $M_0$  and  $\theta$ . When in addition  $\epsilon$  and the reaction constants  $R$ ,  $\mu$  and  $\gamma$  are known, the solutions for  $c_r(x, t, v)$  of sections 3.1 and 3.2 can be applied. Further specification of  $\mu_{1n}$  and  $\sigma_{1n}$  defines  $\rho(v)$ , thus allowing  $\hat{c}_r$  to be evaluated by numerical quadrature of [62].

For purposes of field calibration involving a large number ( $n$ ) of random observations for  $c_r$  at fixed  $x$  and  $t$  over the areal extent of the flow region, we may take  $\rho(v) = 1/n$  in [62] for each observation  $c_{r1}$ . This discrete case gives

$$\hat{c}_r = \frac{1}{n} \sum_{i=1}^n c_{r1} \quad [67]$$

which is simply the arithmetic average of the observed local resident concentrations. When observations are not available at fixed  $x$  and  $t$ , it may be feasible to interpolate between given  $x$  and  $t$  graphically or by means of smoothing functions prior to averaging.

### 5.2. Solution for $\hat{c}_f$ .

Let us define a field-scale flux concentration  $\hat{c}_f$  in the same fashion as introduced earlier (Eq. 6) at the "local" level:

$$\hat{c}_f = \frac{\langle J \rangle}{\langle q \rangle} \quad [68]$$

where  $\langle J \rangle$  and  $\langle q \rangle$  respectively represent the time-dependent solute mass flux and volumetric fluid flux across a plane perpendicular to the velocity field. From the definition of  $c_f$  at the local continuum level (Eq. 6), we may replace  $J$  by  $qc_f$ . With the assumption that  $\theta$  is deterministic, [68] becomes then

$$\hat{c}_f = \frac{\langle vc_f \rangle}{\langle v \rangle} \quad [69]$$

which in turn may be expanded to give

$$\hat{c}_f = \frac{\int_0^{\infty} vc_f \rho(v) dv}{\int_0^{\infty} v \rho(v) dv} \quad [70]$$

The denominator in [70] can be obtained analytically for the log-normal distribution function as indicated by [58] and [59]. The numerator can be evaluated numerically for given  $\rho(v)$  and  $c_f$ , with the latter obtained analytically from the solutions in sections 3.3 and 3.4. Field-scale initial and boundary conditions for  $\hat{c}_f$  are analogous to those imposed on  $\hat{c}_r$ .

From Eq. [70], it follows that  $\hat{c}_f$  differs from the expected value  $\langle c_f \rangle$ , which for deterministic  $\theta$  is given by



$$\langle c_f \rangle = \frac{\int_0^{\infty} c_f \rho(v) dv}{\int_0^{\infty} \rho(v) dv}. \quad [71]$$

This behavior is in contrast to that for  $\hat{c}_r = \langle c_r \rangle$  and has important implications to model calibration via measurements of local  $c_f$ -values. For the general case of variable water content  $\theta$ , random sampling leads to the discrete form for  $\hat{c}_f$ :

$$\hat{c}_f = \frac{\sum_{i=1}^n (q c_f)_i}{\sum_{i=1}^n q_i}. \quad [72]$$

Because local values for  $q$  are not easily obtained experimentally, evaluation of the field-scale flux-weighted average concentration (Eq. 72) is extremely difficult. In general it is therefore advisable to use resident concentration measurements for calibration of regional transport models. The main advantage of the solution for  $c_f$  lies in the fact that it leads directly to an estimate for the field-scale solute flux  $\langle J \rangle(x,t)$  for given  $\langle q \rangle$ .

## 6. COMPUTER PROGRAM DESCRIPTION

A FORTRAN IV computer program (CXTFIT) was written that permits one to fit any of the previously discussed analytical solutions for  $c_r$ ,  $c_f$ ,  $\hat{c}_r$  or  $\hat{c}_f$  to observed concentration distributions as a function of time and/or distance. The curve-fitting method uses the maximum neighborhood method of Marquardt (1963) to minimize the sums of squares of the residuals between observed and calculated concentrations. A detailed description of the method is given by Daniel and Wood (1973). Two previous versions of this same curve-fitting program were used by van Genuchten (1980, 1981) to analyze breakthrough curves in time. Program CXTFIT can be applied also to spatial concentration distributions (e.g., as obtained from

sectioned soil columns or from field core data) as well as to simultaneous spatial and temporal distributions (e.g., as obtained from replicated cores sectioned at different times). The basic format of the earlier programs, including notation and set-up of the input data file, has been maintained as much as possible. Aside from the least-squares regression analysis, the program can be used also to predict spatial and temporal concentration profiles for given coefficient values using one of the analytical solutions listed in this report. The least-squares inversion part of the program will be bypassed in that case. A listing of CXTFIT is given in Appendix E.

A list of the most important parameters of CXTFIT is presented in Appendix A. Appendix B gives instructions for setting up data input files, while Appendix C lists selected input files used for various examples to be discussed later. The computer output file for these examples is given in Appendix D.

The computer model itself consists of a MAIN section, six subroutines (LIMIT, MATINV, MODEL, MOD12, MOD34 and ROMB) and four functions (CAVG, CCO, EXF and GOLD). Input and output instructions and most of the parameter optimization calculations are carried out in MAIN. Of the four functions, EXF evaluates the product of the exponential function ( $\exp$ ) and the complementary error function ( $\text{erfc}$ ). The function CAVG calculates the arguments of the integrals in [62] and [70] for the regional transport model. The function CCO calculates the argument of the integral in [51] for the two site/two-region model, while GOLD evaluates Goldstein's J-function that appears in that same equation.

Of the six subroutines, MATINV performs a matrix inversion needed for the least-squares analysis. Subroutine MODEL performs coefficient assignments and routes execution to the appropriate subroutine for evaluation of one of the analytical solutions. Subroutine MOD12 evaluates all analytical solutions for deterministic models involving linear equilibrium adsorption while MOD34

similarly evaluates the analytical solutions for the two-site/two-region non-equilibrium models. The stochastic models also call on subroutine MOD12 to evaluate local concentration distributions.

Numerical integrations required for the two-site/two-region and stochastic models are carried out in subroutine ROMB which performs an eighth-order Romberg quadrature on a log-transformed interval. A relative error (STOPER) of  $5 \times 10^{-5}$  is currently used in ROMB. For most of our problems, convergence was reached with only 16-32 quadrature points. This high degree of efficiency was achieved in part by using a logarithmic transformation and in part by judiciously limiting the integration interval to a small region within which the argument of the integral is not negligible. This approach concentrates the quadrature points in the region of greatest sensitivity.

The modified lower (T1) and upper (T2) integration limits employed for the two-site/two-region models are (see also Eq. [51])

$$T1 = \text{MAX}(0., A) \quad [73]$$

$$T2 = \text{MIN}(TT, B) \quad [74]$$

where

$$A = \beta R z + \frac{40 \beta R}{P} \left[ 1 - \left( 1 + \frac{Pz}{20} \right)^{1/2} \right] \quad [75]$$

$$B = \beta R z + \frac{40 \beta R}{P} \left[ 1 + \left( 1 + \frac{Pz}{20} \right)^{1/2} \right]. \quad [76]$$

This modified integration interval was obtained by limiting integration to that region where the exponential function in [56] exceeds  $\exp(-20)$ . In instances where a zero lower limit remains, a five point Gaussian quadrature is carried out in MOD34 on the interval from 0 to  $T2 \times 10^{-4}$  before passing to ROMB. This procedure was found to be more accurate and efficient than a fixed 40-point Gaussian quadrature scheme previously applied to the untransformed interval (van Genuchten, 1981).

To increase computational efficiency, we also narrowed the integration limits for the stochastic transport model. Computation of these limits in subroutine LIMIT involves incremental searching with convergence by the Newton-Raphson method. Reasonable initial estimates for the narrowed limits, derived from known properties of the probability density function and also using the relationship between initial breakthrough and the local Peclet number  $Pz=x/\epsilon$ , facilitated rapid convergence to suitable limits. For most problems that we investigated, subroutine LIMIT required only 10-20 solutions for local concentration values, while subsequent numerical integration in subroutine ROMB required 16-32 quadrature points.

Table 1 lists salient features of the eight models, each distinguished by a different value for the input parameter MODE. Model parameters listed in the table represent the variable coefficients for each model, i.e., coefficients that potentially can be fitted to observed data. The deterministic equilibrium models (MODE = 1,2) and the two-site/two-region non-equilibrium models (MODE = 3,4) each contain six parameters, while the stochastic models (MODE = 5,6,7 or 8) contain seven parameters. Note that  $t_o$  is a fixed input parameter for Models 5 and 6, while for Models 7 and 8 the ratio  $M_o/\theta$  must be entered. Also note that for the stochastic models the probability distribution function for  $v$  is described by  $\langle v \rangle$  and  $\sigma_{ln}$  rather than by  $\mu_{ln}$  and  $\sigma_{ln}$ . This mode of input was deemed more useful because it is much more likely that one has an estimate for the mean field velocity  $\langle v \rangle$  rather than  $\mu_{ln}$ , the latter quantity corresponding to the median of the untransformed velocity distribution. For example,  $\langle v \rangle$  can at least in principle be estimated from a simple water balance. Due to the log-transformation,  $\sigma_{ln}$  represents a dimensionless index of the dispersion of the velocity distribution.

Of the six coefficients in Models 1-4, at most five are mutually independent. Thus, for those models a maximum of only five parameters can be fitted simultaneously to the observed data. This

Table 1. Characteristics of models designated in program CXTFIT

Model number (MODE)	Model Type	Concentration detection mode	Model parameters	Report section
1	Deterministic linear equilibrium adsorption	Resident	$v, D, R, t_0, \mu, \gamma$	3.1
2	Same as 1	Flux	Same as 1	3.2
3	Two-site/two-region	Resident	$v, D, R, t_0, \beta, \omega$	4.1
4	Same as 3	Flux	Same as 3	4.2
5	Stochastic linear equilibrium adsorption with constant $t_0$	Resident	$\langle v \rangle, \epsilon, R, t_0, \mu, \gamma, \sigma_{1n}$	5.1
6	Same as 5	Flux	Same as 5	5.2
7	Stochastic linear equilibrium adsorption with constant $M_0$	Resident	$\langle v \rangle, \epsilon, R, M_0/\theta, \mu, \gamma, \sigma_{1n}$	5.1
8	Same as 7	Flux	Same as 7	5.2

dependency of at least one of the coefficients follows immediately by considering one of the transport equations, for example Eq. [11] for the linear equilibrium adsorption model. It is apparent that dividing this equation through by a constant permits one of the coefficients to be eliminated. Consequently, at least one of the coefficients  $v$ ,  $D$ ,  $R$ ,  $\mu$  or  $\gamma$  in [11] must be known independently. Because the values of  $D$ ,  $\mu$  and  $\gamma$  generally are not easily measured independently, in practice either  $v$  or  $R$  (or both) must be known beforehand. For non-adsorbing chemicals,  $R = 1$ , and the pore-water velocity  $v$  can be fitted to the data if so desired--a useful procedure when poor estimates for  $v$  are available, e.g., because of uncertainty about the effective water content in the system owing to

negative adsorption (assuming the flux  $q$  to be known), or because of experimental problems. For adsorbing chemicals,  $R$  can at least in principle be estimated using batch-equilibration techniques, thus allowing  $v$  to be estimated from tracer experiments. On the other hand,  $R$  is often obtained also by directly fitting this coefficient to experimental data. In that case it is imperative that an accurate independent estimate for  $v$  be obtained first.

In addition to the mutual dependency of  $v$  and  $R$ , we also observed in some cases significant interactions between the coefficients  $\mu$ ,  $\gamma$  and  $t_0$  (see examples 1 and 2 of section 7). This behavior is not surprising since these three parameters, together with  $R$ , determine the total amount of solute that will be found in the porous medium (or in the effluent). Similar interactions also occur for Models 5-8, in particular between  $\langle v \rangle$  and  $R$  and between  $\mu$ ,  $\gamma$  and  $t_0$  (Models 5, 6) or  $\mu$ ,  $\gamma$  and  $M_0/\theta$  (Models 7, 8). To avoid these type of uniqueness problems, we recommend to limit as much as possible the number of coefficients that will be fitted to observed data. Limiting the number of unknown coefficients is particularly important when experimental and medium variabilities create uncertainty in the data.

Appendix B gives instructions for setting up the data input file. The first card specifies the number of examples that will be executed. The second card specifies the model number (MODE) as indicated in Table 1 and a data input code (NDATA) specifying whether the next example uses the same  $c(x,t)$  data as in the previous example (but for a different model number), or whether new data are to be read in. This second card also gives the input values for NREDU (a code specifying whether or not the input concentrations and times are in dimensionless form), MIT (maximum number of iterations allowed during execution), NOB (number of observed data points), NSKIP (a code indicating whether the following example in the input data file is to be executed or skipped--a convenient feature to reduce editing of large input

files), and NPRT (a code that allows one to omit input concentration data from the computer printout). After two information cards (cards 3 and 4), the input file needs a list of names for the six coefficients, BI(I), on card 5. Initial estimates for these coefficients are given on card 6. Table 1 specifies the exact sequence of the names and estimates that must be provided. Card 7 lists 6 indices, INDEX(I), that stipulate whether a certain coefficient is to be held constant (INDEX = 0) or fitted to the data (INDEX = 1). If a coefficient is assumed to be known, the initial estimate of this coefficient on card 6 will remain unchanged during the least-squares analysis. Card 8 lists CI (the initial concentration) and CO (the inlet pulse concentration). Finally, the remaining cards are used to enter the observed tracer concentration data as a function of observed distances, x, and times, t (see Appendix B for required formats).

Experimental results from laboratory soil columns are often presented in reduced form as a function of the number of pore volumes T (=vt/L) leached through the column. Dimensionless effluent concentrations,  $\bar{c}$ , for pulse-type injections are usually expressed in the form

$$\bar{c}(z,T) = \frac{c(z,T) - C_1}{C_0 - C_1} \quad [77]$$

where  $c(z,T)$  is the observed concentration. For effluent data,  $c$  in [77] represents a flux concentration. A similar reduction into dimensionless concentrations may be carried out also for resident concentration values when appropriate. As mentioned before, an option in the program allows reduced data to be accepted as input. By setting input code NREDU = 1, all concentrations are read in as reduced values but times are still considered to be actual values. Setting NREDU = 2 stipulates the input of reduced concentrations ( $\bar{c}$ ) as well as reduced times (T). For Models 1-4, the value of  $t_0$  in the input file must then be replaced by its reduced counterpart  $T_0 =$

$vt_0/L$ . Finally, when  $NREDU = 2$ , the program does not allow  $v$  to be fitted to the data, which means that the value of  $v$  must be specified on input. For the stochastic models ( $MODE = 5-8$ ), time reduction may be achieved by substituting  $\langle v \rangle$  for  $v$ , provided  $\langle v \rangle$  is then also fixed on input.

The value of  $L$  used in defining several dimensionless variables, in particular  $\omega$  for Models 3 and 4, is defined internally in the program as the maximum value in the  $x$ -array. Because of the scaled form of [41] and [42], it is apparent from the definitions of the dimensionless variables that if values for  $z$ ,  $T$  and  $T_0$  are entered in lieu of  $x$ ,  $t$  and  $t_0$ , and provided  $v$  is fixed at unity, then the coefficient that occupies the  $D$ -field is in actuality  $1/P$ , while the other coefficients ( $\beta$  and  $R$ ) retain their original meaning. The same implicit reduction can be applied also to Models 1 and 2 to obtain  $1/P$  in lieu of  $D$ . More generally, any one of the coefficients  $v$ ,  $D$ ,  $R$ ,  $\mu$  or  $\gamma$  in Models 1-4 can be arbitrarily fixed at unity on input while the other coefficients are entered as usual. Because of the linearity of [11] and [21], the input or fitted parameters then take on the meaning  $v/a$ ,  $D/a$ ,  $R/a$ ,  $\mu/a$  or  $\gamma/a$  where  $a$  is the actual value of the fixed coefficient. These reductions of the input data can be imposed irrespective of the value of  $NREDU$  and add more flexibility when applying the computer code to different types of data sets.



## 7. APPLICATIONS

Several examples have been run with CXTFIT to demonstrate its accuracy and versatility. These include a number of hypothetical examples used primarily to test the inversion method as well as several experimental data sets, thus illustrating practical applications of the program.

The first example (1a) simply calculates hypothetical concentration distributions for a reactive solute exhibiting linear equilibrium adsorption, zero-order production, and first-order decay. The analysis is deterministic with medium coefficients  $v=25$  cm day<sup>-1</sup>,  $D=100$  cm<sup>2</sup> day<sup>-1</sup>,  $R = 2.5$ ,  $\mu=0.25$  day<sup>-1</sup> and  $\gamma=0.5$   $\mu\text{g cm}^{-3}$  day<sup>-1</sup>. The initially solute-free medium ( $C_i=0$ ) is subjected to a pulse input of  $C_o=100$   $\mu\text{g cm}^{-3}$  for a duration  $t_o$  of 5 days. The predicted resident concentrations for depths 0-100 cm at  $t=5$  and 10 days were calculated by setting MIT=0 in the input file. The input and output files are shown in Appendices C and D, respectively. Results for example 1a are plotted in Figure 1.

In example 1b, the  $c_r(x,t)$  data from example 1a for both  $t=5$  and 10 days were used as input to the nonlinear regression program, thus providing a test of the parameter estimation method. The parameters  $D$ ,  $R$ ,  $\mu$  and  $\gamma$  were estimated from initial input estimates of 1.0 for all coefficients. The pore water velocity  $v$  was assumed to be known independently (at least one non-zero coefficient must be known). The fitted parameter values were essentially equal to the true values (Table 2), with residuals of the concentrations all being less than the roundoff level of the input data (Appendix D). Results of nearly the same accuracy were obtained when the inversion was restricted to only the  $t=5$  days data (example 1c, Table 2). Attempts to estimate  $t_o$  in addition to the other four parameters indicated a large plateau in the response surface. No convergence was obtained with a stop criterion (STOPCR) of 0.0005 and with a maximum number of trials with no residual decrease within each iteration (MAXTRY) set at 50. A larger value for MAXTRY probably

Table 2. Comparison of fitted transport parameters for example 1.\*

Example	D	R	$\mu$	$\gamma$
1a True values for $c_r$	100.000	2.5000	.2500	.5000
1b 22 point fit to $c_r$	100.062	2.4990	.2499	.4999
1c 11 point fit to $c_r$	100.280	2.5006	.2496	.4971
1d 11 point fit to $c_f$	85.6	2.52	.34	.76
1e 11 point fit to $c_f$	161.7	3.00	(.25)	(.50)

\*D in  $\text{cm}^2 \text{ day}^{-1}$ ,  $\mu$  in  $\text{day}^{-1}$ ,  $\gamma$  in  $\mu\text{g cm}^{-3} \text{ day}^{-1}$ ; values in parenthesis were fixed on input.

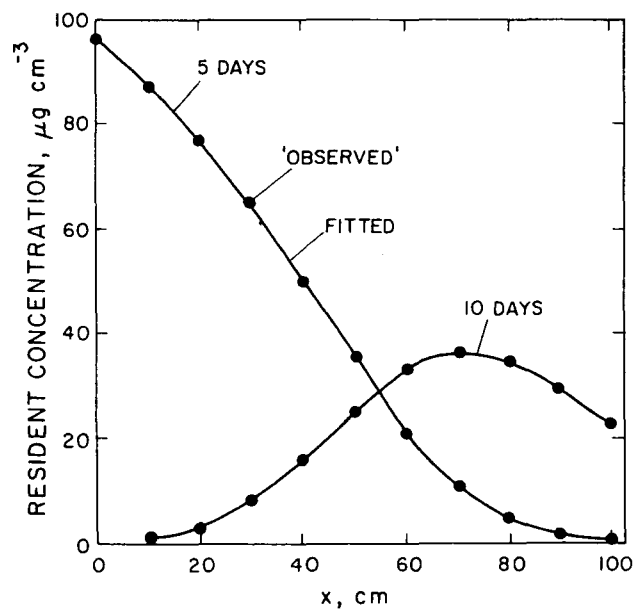


Figure 1. Hypothetical and fitted resident concentration distributions for deterministic linear equilibrium adsorption (example 1 using Model 1).

would have facilitated estimation of  $t_0$  also. However, in most cases  $t_0$  will be known, rendering its estimation unnecessary.

Boundary conditions appropriate for the analysis of flux concentrations have frequently been used to interpret data assumed to represent resident concentrations. To evaluate the effect of fitting the flux concentration solution to observed resident concentration data, the parameters in Model 2 for  $c_f$  were fitted to the  $t=5$  data for  $c_r$ . As shown by example 1d in Table 2, the fitted coefficient values in this case do not compare well with the "true" values. With  $\mu$  and  $\gamma$  fixed at their correct values, a two-parameter fit greatly overestimated  $D$  and  $R$  (example 1e, Table 2). This overestimation is a characteristic occurrence when resident concentrations are misinterpreted as flux concentrations (Parker and van Genuchten, 1984). The converse will be found when flux concentrations are interpreted as resident concentrations.

Example 2a predicts hypothetical flux concentrations for the two-site/two-region non-equilibrium model (Model 4) using as parameter values  $v=10$  cm day<sup>-1</sup>,  $D=1.5$  cm<sup>2</sup> day<sup>-1</sup>,  $R=3.0$ ,  $\beta=0.3$  and  $\omega=2.5$  (based on  $L=40$  cm). A 2.5 day ( $t_0$ ) long pulse of concentration  $C_0=500$   $\mu\text{g cm}^{-3}$  is applied to an initially solute-free medium ( $C_1=0$ ). Predicted concentrations at  $x=20$  and  $40$  cm from  $0$  to  $12$  days are shown in Figure 2. In example 2b, these predicted concentrations are used in the inversion program to estimate  $D$ ,  $R$ ,  $t_0$ ,  $\beta$  and  $\omega$ , using as starting values for these coefficients  $10$  cm<sup>2</sup> day<sup>-1</sup>,  $5.0$ ,  $1.0$ ,  $0.5$  and  $1.0$ , respectively. In  $10$  iterations the program converged to the correct values within a relative error of  $0.0001$  (see Appendix D). Using the concentration data for only one of the depths, the program failed to converge to the correct parameter values when all five parameters were fitted. However, the program converged correctly when  $t_0$  was fixed and the remaining four parameters were estimated from the smaller data sets.

The third example is used to analyze a column tracer experiment reported by Parker (1984). The experiment was conducted

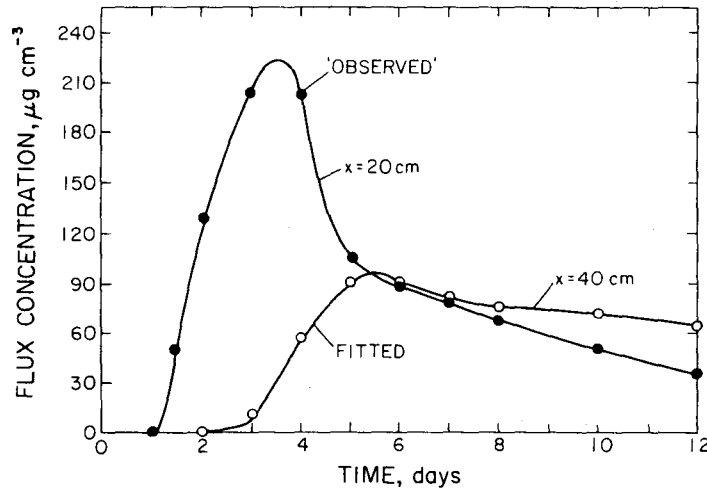


Figure 2. Hypothetical and fitted flux concentration distributions for deterministic two-region/two-site nonequilibrium model (example 2 using Model 4).

on a cylindrical column 52 mm in diameter, 190 mm long, filled with a mixture of sand and a small amount of cement as a binding agent. The column contained a straight 1.7 mm diameter "wormhole" passing axially through the center of the medium. Effluent from the column, subjected to steady saturated flow, was collected subsequent to the addition of 0.65 pore volumes of a bromide tracer to an initially Br-free pore solution. In total about 1.8 pore volumes of effluent were collected. A similar experiment was performed on a replicate column, but in this case the column was sectioned into 19 mm depth intervals immediately following the addition of the bromide pulse. The column sections were extracted to determine the Br resident concentration distribution at the time of sectioning.

The bromide effluent curve (Fig. 3) is plotted in terms of reduced concentrations versus reduced times or pore volumes. The

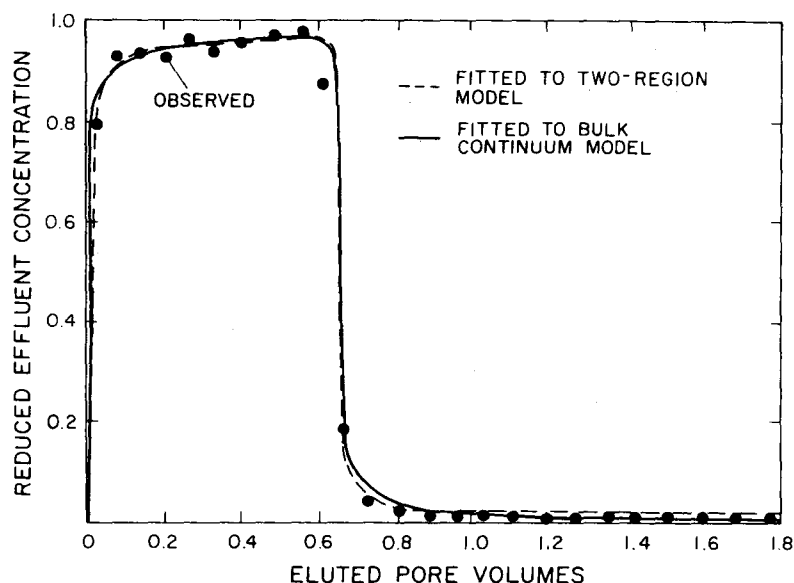


Figure 3. Experimental bromide effluent curve for packed sand with "wormhole", and fitted curves for bulk continuum model (Model 2) and two-region model (Model 4).

mean pore water velocity  $v$  determined from the measured hydraulic flux ( $39.7 \text{ m day}^{-1}$ ) and water content ( $0.371 \text{ cm}^3 \text{ cm}^{-3}$ ) was  $107 \text{ m day}^{-1}$ . Assuming no bromide adsorption (positive or negative),  $R$  was taken to be unity. Effluent concentrations and corresponding times were entered in the program as reduced values  $\bar{c}(T)$ , while also a reduced pulse duration  $T_0$  was used in lieu of  $t_0$  (NREDU=2). Finally, because reduced variables are used and because  $\gamma=\mu=0$ , we may simply set  $C_i=0$  and  $C_0=1$ .

If the porous medium is viewed as a simple continuum, then Model 2 for  $c_f$  should be used to analyze the effluent data. In our example only  $D$  is unknown. The fitted value for  $D$  (see example 3a, Appendix D) was found to be  $8745 \text{ m}^2 \text{ day}^{-1}$ . Alternatively, one may view the bromide experiment as a two-region transport problem, in which case Model 4 should be applied. With  $R=1$ , the values of

$D$ ,  $\beta$  and  $\omega$  in this model were found to be  $300 \text{ m}^2 \text{ day}^{-1}$ , 0.043 and 0.103, respectively. Figure 3 shows that Models 2 and 4 describe the experimental data with comparable accuracy. Hence, there is in this case little advantage in adopting the two-region model with three parameters instead of the much simpler bulk continuum model with only one adjustable parameter.

The importance of distinguishing between resident and flux concentrations may be evaluated by using the fitted value for  $D$ , based on Model 2 for  $c_f$ , to predict with Model 1 the resident concentration distribution ( $c_r$ ) at the time the second column was sectioned. These predicted values are compared in Figure 4 with the

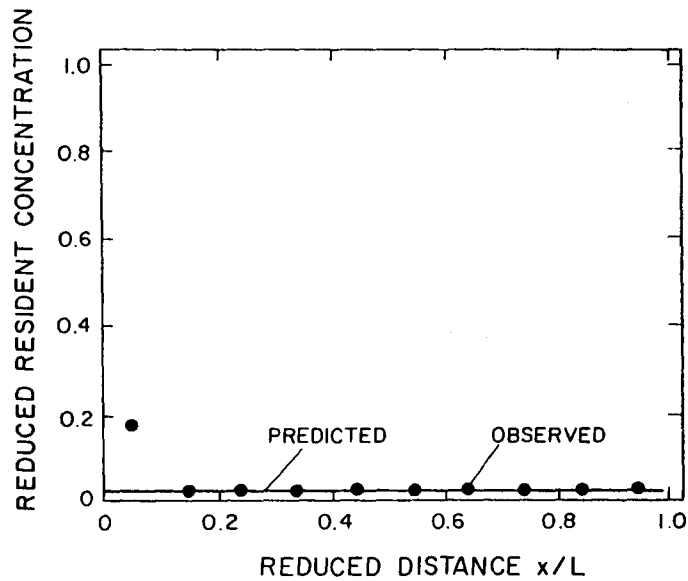


Figure 4. Experimental resident concentration distribution in "wormhole" column and predicted distribution from Model 1 using  $D$  fitted to the effluent data of Figure 3 with Model 2.

measured resident values representing mean concentrations in each 19 mm section. The correspondence is very close except in the uppermost section in immediate contact with the influent solution. The discrepancy here may be expected owing to assumptions implicit in the macroscopic description of the boundary region at or near  $x=0$  (Parker, 1984). In any case, this example emphasizes the importance of distinguishing between resident and flux concentrations. The data show that the reduced flux-averaged effluent concentration was about 0.98 at the time the column was sectioned, while the resident concentration in the column near the exit was only 0.02. Failure to realize that the effluent concentration is not equal to the resident concentration at the exit would make it impossible to fit  $D$  with Model 1 to the effluent data using the measured value for the pore water velocity. A two-parameter fit of Model 1 with variable  $v$  and  $D$  would actually lead to an estimate for the pore-water velocity that is about 1000 times greater than the measured value, thus suggesting that a large fraction of the pore space was effectively excluding the bromide tracer. This conclusion is unwarranted and unnecessary when the transport model is chosen to correspond to the proper solute detection mode.

Example 4 considers the movement of boron through Glendale clay loam (Exp. 3-1 of van Genuchten, 1974). A boron tracer pulse of duration  $t_0=5.06$  days and having a concentration of  $C_0=20 \mu\text{g cm}^{-3}$  was leached through an initially solute-free 30 cm long column. The measured pore water velocity was  $38.5 \text{ cm day}^{-1}$ . Reduced concentration and times were used ( $\text{NREDU}=2$ ); hence,  $T_0=vt_0/L$  was entered in lieu of  $t_0$  as a known input parameter for the pulse duration. Assuming non-equilibrium adsorption and noting that flux concentrations must be used to describe effluent data, we employed Model 4 to fit the coefficients  $D$ ,  $R$ ,  $\beta$  and  $\omega$ . The final parameter estimates and the sum of squared residuals (SSQ) of the observed versus fitted concentrations for two different sets of initial estimates (examples 4a,b) are shown in Table 3. Results of example 4a, which yielded

the lowest SSQ, are compared with the experimental data in Figure 5. Note that the initial estimates for example 4b yielded fitted coefficient values with a higher SSQ using the currently set stop criteria (STOPCR and MAXTRY). Clearly, the program can converge to local minima with erroneous final estimates. Critical to correct and relatively rapid convergence is a good initial estimate of the product  $\beta R$ . For negligible dispersion ( $D=0$ ), this product is equivalent to the number of pore volumes  $T$  at which the tracer initially appears in the effluent (van Genuchten and Cleary, 1979). From the experimental data in Figure 5, we may estimate the product  $\beta R$  to be about 2.0 (initial boron breakthrough) to 3.0 (the value of  $T$  at a reduced concentration of approximately 0.5). For not too wild initial guesses for  $D$  and  $\omega$ , we always obtained convergence to the final estimates of example 4a in Table 3 when the initial estimates for  $\beta$  and  $R$  were such that  $1.5 < \beta R < 4$ . For other estimates, the program sometimes converged to the same estimates, sometimes converged to estimates with higher SSQ-values, or sometimes diverged. Consequently, a reasonable initial guess for the product  $\beta R$  is essential when applying the two-site/two-region

Table 3. Fitted parameter values and associated residual SSQ's for example 4 for two sets of initial estimates.\*

Example	D	R	$\beta$	$\omega$	SSQ
4a. Initial values	2.00	10.00	.200	.200	
Final values	47.7	4.30	.600	.424	.053
4b. Initial values	1.0	1.00	.500	.200	
Final values	216.	3.58	.564	14.2	.142

\*  $D$  in  $\text{cm}^2 \text{ day}^{-1}$ ; other parameters are dimensionless.



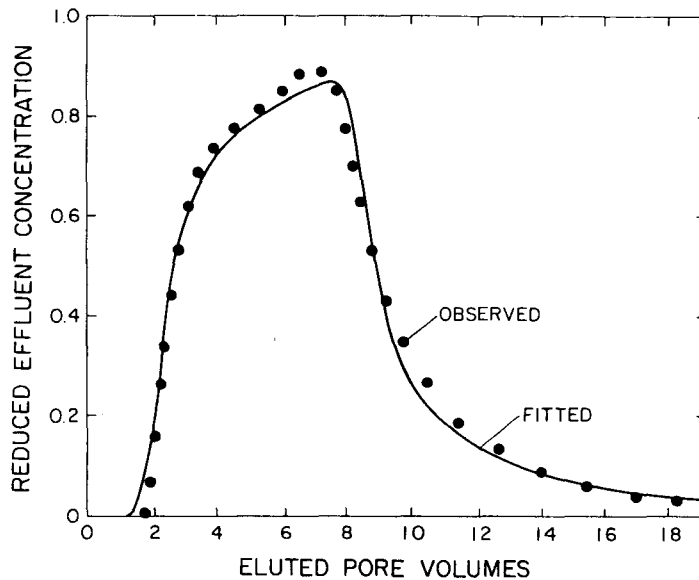


Figure 5. Experimental and fitted boron effluent curves for Glendale clay loam (example 4 using Model 4).

non-equilibrium model. We finally note that the fitted parameter values of example 4a (Table 3) obtained here with a Romberg integration scheme are the same as those obtained previously with a 40-point Gaussian quadrature scheme (van Genuchten, 1981).

Example 5 involves again a hypothetical case and deals with the stochastic regional transport model applied to a field subject to a mean velocity  $\langle v \rangle$  of  $25 \text{ cm day}^{-1}$  with a standard deviation for the logarithmic probability density distribution ( $\sigma_{\ln}$ ) of 1.37. The initially solute-free medium ( $C_1=0$ ) is subjected to a uniform dose of solute ( $M_0$ ) of  $10,000 \text{ } \mu\text{g cm}^{-2}$ , which for  $\theta=0.5$  yields the input parameter  $\text{SOLOAD} \equiv M_0/\theta = 20,000 \text{ } \mu\text{g cm}^{-2}$ . We assume the inlet solute concentration ( $C_0$ ) to be  $1,000 \text{ } \mu\text{g cm}^{-3}$  and take  $\epsilon=10 \text{ cm}$ ,  $R=5.0$  and  $\mu=\gamma=0.0$ . Figure 6 shows calculated concentration profiles

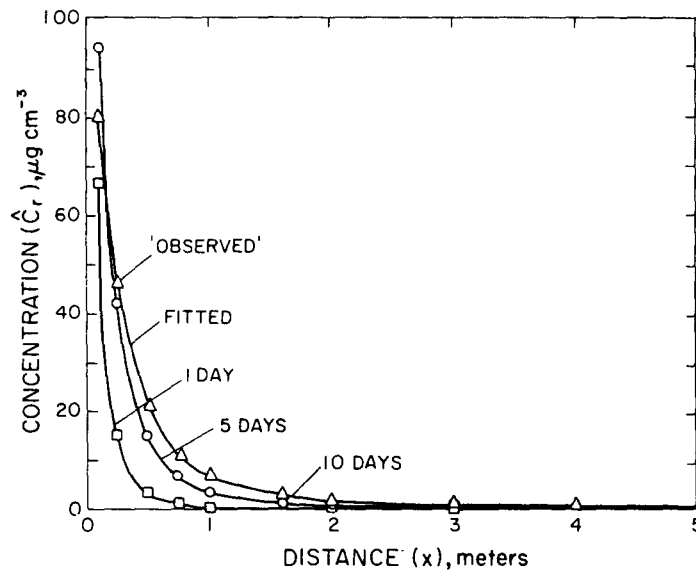


Figure 6. Hypothetical and fitted resident concentration distributions for stochastic transport model (example 5 using Model 7).

(Model 4;  $MIT=0$ ) from 0 to 500 cm and for  $t=1, 5$  and 10 days (example 5a). Using 30 input data points for the combined times and initial guesses of  $10 \text{ cm day}^{-1}$  for  $\langle v \rangle$  and unity for the other parameters, the inversion program yielded estimates for  $\langle v \rangle$ ,  $\epsilon$ ,  $R$  and  $\sigma_{1n}$  with a maximum error of 0.0001 from the correct values (example 5b, Appendix D). The program converged to incorrect parameter values when in addition  $M_0/\theta$  was fitted to the data. Execution terminated with parameter values fixed on a very flat response surface with SSQ still large. In such cases it is

sometimes possible to move off the plateau in the response surface by decreasing STOPCR and increasing MAXTRY. In less troublesome circumstances, greater computational efficiency will be obtained, however, when STOPCR is relatively large and MAXTRY relatively small (see also our comments on uniqueness problems in the previous section).

The final example considers a case of field-scale stochastic transport. Unfortunately, field measurements suitable for testing and calibrating stochastic transport models are scarce. Jury et al. (1982) reported areally-averaged concentrations over a 0.64 ha field subjected to a bromide pulse of fixed duration and subsequently leached under transient hydraulic conditions. The stochastic model presented here does not strictly apply to transient conditions. However, by invoking some approximate transformations of temporal variables, the model can be extended in an approximate manner also to transient conditions.

Let us define a time-averaged flux  $\langle q \rangle^*$ , a time-averaged water content  $\langle \theta \rangle^*$ , and a time-averaged pore water velocity  $\langle v \rangle^*$  as follows:

$$\langle q \rangle^* = \frac{1}{t_m} \int_0^{t_m} \langle q \rangle(\tau) d\tau \quad [78]$$

$$\langle \theta \rangle^* = \frac{1}{t_m} \int_0^{t_m} \langle \theta \rangle(\tau) d\tau \quad [79]$$

$$\langle v \rangle^* = \frac{\langle q \rangle^*}{\langle \theta \rangle^*} \quad [80]$$

where  $\langle q \rangle$  and  $\langle \theta \rangle$  respectively represent the actual time-dependent areally-averaged fluxes and water contents over the time interval  $t=0$  to  $t_m$  of interest. We assume  $\langle q \rangle^*$  and  $\langle \theta \rangle^*$  to be independent of  $x$ . For  $\langle q \rangle^*$  this assumption implies that there are no fluid sources or sinks in the flow region, while  $t_m$  must be relatively large. For  $\langle \theta \rangle^*$ , the additional constraint of medium homogeneity in the  $x$

direction is implied. If these assumptions are not appropriate and information regarding the variation of  $\langle q \rangle$  and  $\langle \theta \rangle$  with  $x$  is available, an approximate transformation of the space coordinate may be imposed in the manner of Bresler and Dagan (1981):

$$x^* = \frac{\langle \langle v \rangle^*(x) \rangle}{\langle v \rangle^*(x)} x \quad [81]$$

where  $\langle v \rangle^*(x)$  is the temporally- and areally-averaged velocity at distance  $x$ , while  $\langle \langle v \rangle^*(x) \rangle$  represents the mean of  $\langle v \rangle^*$  over all  $x$ . We will not impose the latter transformation but instead take  $\langle v \rangle^*$  as constant and calculate  $\langle q \rangle^*$  by means of a water balance at  $x=0$ . Transformation of the time variable itself is accomplished with

$$t^* = \frac{\int_0^t \langle q \rangle(\tau) d\tau}{\langle q \rangle^*} \quad [82]$$

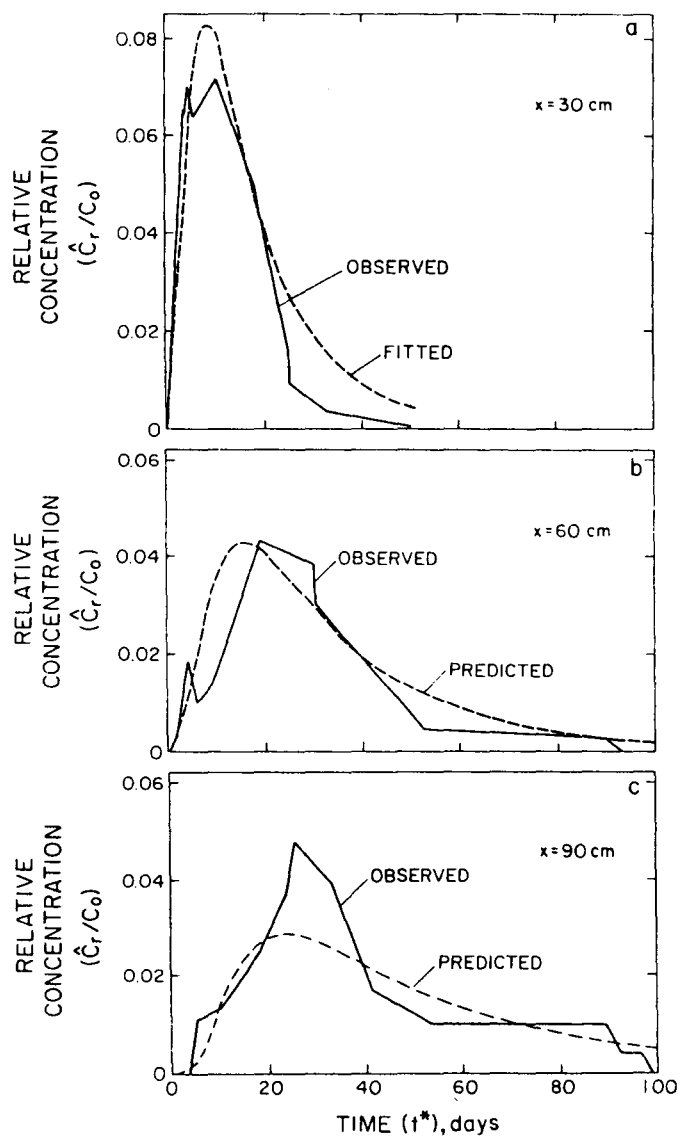
where the numerator represents the cumulative flux until time  $t$ . To implement this model for temporally transformed data, we need to estimate  $t^*$  and  $\langle v \rangle^*$  instead of  $t$  and  $\langle v \rangle$ . Averaging the data of Jury et al. (1982) over a period of 100 days yields  $\langle q \rangle^* = 5.9$  mm day<sup>-1</sup>. From  $\langle q \rangle^*$  and the reported values of the cumulative net amount of applied water versus areally-averaged concentrations, we may calculate the times  $t^*$  that correspond to these measured concentrations. Since  $\langle \theta \rangle^*$  is not known,  $\langle v \rangle^*$  cannot be calculated directly. However, a reasonable first guess of  $\langle v \rangle^*$  for the inversion program can be obtained by using  $\langle \theta \rangle^* = 0.2$ . Application of the tracer was accomplished by means of a uniform 10-mm irrigation over the entire field. By regarding the measured concentrations as resident values, we may use Model 5 to analyze the data. The input value of  $t_0$  for that purpose was replaced by the transformed pulse time  $t_0^*$ , which according to [82] equals  $10/\langle q \rangle^* = 1.69$  days. We further assume  $R=1.0$  and  $\mu=\gamma=0.0$  for bromide transport. The program can now be used to calculate  $\langle v \rangle^*$ ,  $\epsilon$  and  $\sigma_{1n}$ .

Using 10 values for  $\hat{c}_r$  that were estimated from observations at  $x=30$  cm from  $t^*=0$  to 50 days, the fitted coefficient values were found to be  $\langle v \rangle^* = 30.5 \text{ mm day}^{-1}$ ,  $\epsilon = 1.0 \text{ mm}$  and  $\sigma_{1n} = 0.800$  (example 6a, Table 4). The results indicate that the estimated value of  $\epsilon$  includes zero within the 95% confidence limits. To further investigate the implications of this uncertainty in the estimation of local dispersivity, we fitted  $\langle v \rangle^*$  and  $\sigma_{1n}$  to the data while fixing  $\epsilon$  at various values. The results (examples 6b-6e, Table 4) show that the fitted values of  $\langle v \rangle^*$  and  $\sigma_{1n}$  are little affected by variations in  $\epsilon$  between 0.1 and 10.0 mm. Much larger values of  $\epsilon$ , however, cause SSQ to increase while  $\sigma_{1n}$  decreases to compensate for the high local dispersion. It is apparent that local-scale dispersion is small compared to the effects of areal velocity variations on resultant field-scale distributions. Noting that the program requires  $\epsilon > 0$ , it is therefore justifiable to fix  $\epsilon$  in this instance at some small value. If the stochastic model is forced to degenerate to the deterministic monocontinuum model by fixing  $\sigma_{1n} = 0$  (example 6f, Table 4), the best fit for  $\epsilon$  is 123 mm, yielding a markedly higher SSQ than when  $\sigma_{1n} \neq 0$ .

Table 4. Fitted model parameters for example 6.\*

Example	$\epsilon$	$\langle v \rangle^*$	$\sigma_{1n}$	SSQ
6a	$1.0 \pm 227.$	$30.5 \pm 21.0$	$.800 \pm .943$	.0005177
6b	(.1)	$30.5 \pm 1.8$	$.803 \pm .060$	.0005182
6c	(1.)	$30.5 \pm 1.8$	$.800 \pm .060$	.0005177
6d	(10.)	$29.7 \pm 1.7$	$.763 \pm .063$	.0005182
6e	(100.)	$24.7 \pm 1.6$	$.370 \pm .143$	.00056
6f	$123. \pm 22.$	$23.6 \pm 1.4$	(0.0)	.00061

\* values  $\pm$  the standard error; units of  $\epsilon$  in mm and  $\langle v \rangle^*$  in  $\text{mm day}^{-1}$ ; values in parenthesis were fixed on input.



Observed and fitted concentration-time curves for the 30 cm depth are plotted in Figure 7a. Using the coefficient values fitted to the 30 cm depth data (example 6a),  $\hat{c}_r(t^*)$  distributions were calculated for depths of 60 cm and 90 cm. Figures 7b and 7c compare these predicted concentrations with the measured values. The tendency for the predicted time of maximum concentration to lag somewhat behind the measured peak time may be due to a variation in  $\langle \theta \rangle$  and hence in  $\langle v \rangle$  with depth in contrast to the homogeneity assumption invoked in our analysis. Implementation of a coordinate transformation, as previously mentioned, may alleviate this problem. On the other hand, the shift in peak concentration could be attributable also to uncertainties in the measured  $\hat{c}_r$ -values. Owing to the rather small sampling size of 14 for each concentration value, Jury et al. (1982) indicated experimental errors in  $\hat{c}_r$  of about 0.05  $C_0$  units at the 95% confidence level. A more stringent test of this or any other stochastic transport model will require a considerably greater sampling density within the transport region.



Figure 7. Experimental areally-averaged concentrations  $\hat{c}_r$  at three depths as a function of transformed time  $t^*$  for bromide transport in a field soil. (a) Data for 30 cm depth and fitted curve using Model 5; (b) data for 60 cm depth and predicted curve using parameter values fitted to the 30 cm depth data; (c) same as b for the 90 cm depth data.

#### 8. SUMMARY AND CONCLUSIONS

The least-squares inversion method discussed in this report provides a convenient, efficient and accurate means of fitting various transport parameters to observed spatial and/or temporal concentration distributions. Both equilibrium and two-site/two-region type non-equilibrium transport models can be implemented in the program. The linear equilibrium adsorption model also includes terms accounting for zero-order production and/or first-order decay. In addition, a stochastic model is described that can be applied to field-scale problems involving log-normally distributed pore water velocity distributions. This model also includes provisions for zero- and first-order production/decay processes. All models are formulated in terms of both flux-averaged concentrations (applicable to column effluent curves) and volume-averaged concentrations (applicable to in situ or resident concentrations). Several example problems illustrating practical applications of the inversion program are discussed in detail.



## 9. REFERENCES

- Agneessens, J. P., P. Dreze and L. Sine. 1978. Modélisation de la migration d'éléments dans les sols. II. Détermination du coefficient de dispersion et de la porosité efficace. *Pédologie* 27:373-388.
- Amoozegar-Fard, A., D. R. Nielsen and A. W. Warrick. 1982. Soil solute concentration distributions for spatially varying pore water velocities and apparent diffusion coefficients. *Soil Sci. Soc. Am. J.* 46:3-9.
- Bennet, A. and F. Goodridge. 1970. Hydrodynamic and mass transfer studies in packed adsorption columns. 1. Axial liquid dispersion. *Trans. Inst. Chem. Eng.* 48:232-244.
- Biggar, J. W. and D. R. Nielsen. 1976. Spatial variability of the leaching characteristics of a field soil. *Water Resour. Res.* 12:78-84.
- Bresler, E. and G. Dagan. 1981. Convective and pore scale dispersive solute transport in unsaturated heterogeneous fields. *Water Resour. Res.* 17:1683-1693.
- Brigham, W. E. 1974. Mixing equations in short laboratory cores. *Soc. Pet. Eng. J.* 14:91-99.
- Cameron, D. A. and A. Klute. 1977. Convective-dispersive solute transport with a combined equilibrium and kinetic adsorption model. *Water Resour. Res.* 13:183-188.
- Coats, K. H. and B. D. Smith. 1964. Dead-end pore volume and dispersion in porous media. *Soc. Pet. Eng. J.* 4:73-84.
- Daniel, C. and F. S. Wood. 1973. *Fitting Equations to Data.* Wiley-Interscience, New York.
- De Camargo, O. A., J. W. Biggar and D. R. Nielsen. 1979. Transport of inorganic phosphorus in an Alfisol. *Soil Sci. Soc. Am. J.* 43: 884-890.
- De Smedt, F. and P. J. Wierenga. 1979. A generalized solution for solute flow in soils with mobile-immobile water. *Water Resour. Res.* 15:1137-1141.

Elprince, A.M. and P.R. Day. 1977. Fitting solute breakthrough equations to data using two adjustable parameters. *Soil Sci. Soc. Am. J.* 41:39-41.

Fluhler, H. and W. A. Jury. 1983. Estimating solute transport using nonlinear rate-dependent two-site adsorption models. *Special Publ. No 245. Swiss Federal Institute of Forestry.* 48 pp.

Gelhar, L. W., A. L. Gutjahr and R. L. Naff. 1979. Stochastic analysis of macrodispersion in a stratified aquifer. *Water Resour. Res.* 15:1387-1397.

Goldstein, F. R. S. 1953. On the mathematics of exchange processes in fixed columns. I. Mathematical solutions and asymptotic expansions. *Proc. Roy. Soc. London.* 219:151-185.

Hoffman, D. L. and D. E. Rolston. 1980. Transport of organic phosphate in soil as affected by soil type. *Soil Sci. Soc. Am. J.* 44:46-52.

Jury, W. A., L. H. Stolzy and P. Shouse. 1982. A field test of a transfer function model for predicting solute transport. *Water Resour. Res.* 18:369-375.

Kreft, A., and A. Zuber. 1978. On the physical meaning of the dispersion equation and its solution for different initial and boundary conditions. *Chem. Eng. Sci.* 33:1471-1480.

Lapidus, L. and N. R. Amundson. 1952. Mathematics of adsorption in beds. VI. The effects of longitudinal diffusion in ion exchange and chromatographic columns. *J. Phys. Chem.* 56:984-988.

Laudelout, H. and J. E. Dufey. 1977. Analyse numérique des expériences de lixiviation en sol homogène. *Ann. Agron.* 28:65-73.

Le Renard, J. 1979. Description d'une méthode de détermination du coefficient de dispersion hydrodynamique longitudinal en milieu poreux homogène saturé en eau. II. Utilisation d'un modèle mathématique a trois parametres dont un phénoménologique. *Ann. Agron.* 30:109-119.

Lindstrom, F. T. 1976. Pulsed dispersion of trace chemical concentrations in a water saturated sorbing porous medium. *Water Resour. Res.* 12:229-238.

Lindstrom, F. T. and L. Boersma. 1975. A theory of the mass transport of previously distributed chemicals in a water-saturated sorbing porous medium. 4. Distributions. *Soil Sci.* 119:411-420.

Lindstrom, F. T. and M. N. L. Narasimhan. 1973. Mathematical description of a kinetic model for dispersion of previously distributed chemicals in a sorbing porous medium. S.I.A.M., J. Appl. Math. 24:496-510.

Lindstrom, F. T. and W. M. Stone. 1974. On the start up or initial phase of linear mass transport of chemicals in a water saturated sorbing porous medium. 1. S.I.A.M., J. Appl. Math. 26:578-591.

Marquardt, D. W. 1963. An algorithm for least-squares estimation of nonlinear parameters. J. Soc. Ind. Appl. Math. 11:431-441.

Parker, J. C., and M. Th. van Genuchten. 1984. Flux-averaged and volume-averaged concentrations in continuum approaches to solute transport. Water Resour. Res. 20 (in press).

Parker, J. C. 1984. Analysis of solute transport in column tracer studies. Soil Sci. Soc. Am. J. 48 (in press).

Pickens, J. F. and G. E. Grisak. 1981. Modeling of scale-dependent dispersion in hydrogeologic systems. Water Resour. Res. 17:1701-1711.

Popovic, M. and W. D. Deckwer. 1976. Transient behavior of reactors with dispersion and stagnant zones. Chem Eng. J. 11:67-72.

Rao, P. S. C., J. M. Davidson, R. E. Jessup and H. M. Selim. 1979. Evaluation of conceptual models for describing non-equilibrium adsorption-desorption of pesticide during steady flow in soils. Soil Sci. Soc. Am. J. 43:22-28.

Selim, H. M., J. M. Davidson and R. S. Mansell. 1976. Evaluation of a two-site adsorption-desorption model for describing solute transport in soils. In Proceedings of the 1976 Summer Computer Simulation Conference, July 1976, Washington, D.C., pp. 444-448.

Simmons, C. S. 1982. A stochastic-convective transport representation of dispersion in one-dimensional porous media systems. Water Resour. Res. 18:1193-1214.

Tang, D. H., F. W. Schwartz and L. Smith. 1982. Stochastic modeling of mass transport in a random velocity field. Water Resour. Res. 18:231-244.

van Genuchten, M.Th. 1974. Mass transfer studies in sorbing porous media. Ph.D thesis, New Mexico State Univ., Las Cruces. 148 p.

van Genuchten, M. Th. 1980. Determining transport parameters from solute displacement experiments. Research Report No 118, U.S. Salinity Laboratory, Riverside, California. 37 p.

van Genuchten, M. Th. 1981. Non-equilibrium transport parameters from miscible displacement experiments. Research Report No 119, U.S. Salinity Laboratory, Riverside, California. 88 p.

van Genuchten, M. Th. and W. J. Alves. 1982. Analytical solutions of the one-dimensional convective-dispersive solute transport equation. U.S. Dept. of Agriculture, Tech. Bull. No. 1661. 151 p.

van Genuchten, M. Th. and R. W. Cleary. 1979. Movement of solutes in soil: Computer-simulated and laboratory results. In G.H. Bolt, ed., Soil Chemistry, B. Physicochemical Models. Elsevier, Amsterdam. pp. 349-386.

van Genuchten, M. Th. and J. C. Parker. 1984. Boundary conditions for displacement experiments through short laboratory soil columns. Soil Sci. Am. J. 48 (in press).

Villiermaux, J. and W. P. M. van Swaay. 1969. Modèle représentatif de la distribution des temps de séjour dans un réacteur semi-infini à dispersion axiale avec zones stagnantes. Chem. Eng. Sci. 24: 1097-1111.

10. APPENDIX A. List of the most significant variables in CXTFIT (units in brackets,  $L^0$  is dimensionless).

Variable	Definition
BETA	Dimensionless coefficient $\beta$ for Models 3 and 4 [ $L^0$ ].
BETR	Value of $\beta/R$ in Models 3 and 4 [ $L^0$ ].
B(I)	Vector containing estimates of the coefficients: $v, D, R, t_o, \mu$ and $\gamma$ for Models 1 and 2, $v, D, R, t_o, \beta$ and $\omega$ for Models 3 and 4, $\langle v \rangle, \epsilon, R, t_o, \mu, \gamma$ and $\sigma_{ln}$ for Models 5 and 6, or $\langle v \rangle, \epsilon, R, M_o/\theta, \mu, \gamma$ and $\sigma_{ln}$ for Models 7 and 8.
BI(I)	Vector of coefficient names.
C(I)	Concentration for I-th observation corresponding to X(I) and T(I) [ $ML^{-3}$ ].
CI	Uniform initial concentration, $C_i$ [ $ML^{-3}$ ].
CO	Concentration of the inlet solution, $C_o$ [ $ML^{-3}$ ].
D	Dispersion coefficient [ $L^2T^{-1}$ ].
DISP	Dispersivity $\epsilon$ for Models 5-8 [L].
DR	Value of $D/R$ [ $L^2T^{-1}$ ].
INDEX(I)	Index for each coefficient. If INDEX(I) = 0, the coefficient B(I) is known and kept constant; if INDEX(I) = 1, the coefficient is assumed to be unknown and fitted to the data.
MAXTRY	Maximum number of trials allowed within an iteration to find new parameter values that decrease SSQ (currently set at 50 in the program).

MIT	Maximum number of iterations allowed in the least-squares analysis. If MIT=0, the least-squares inversion part is bypassed and the program calculates and prints concentrations for given input values of x, t and the different parameters; dummy values of C(I) and INDEX(I) are then still read in but not used in the program.
MODE	Model number specifying type of transport model and boundary conditions to be used (see Table 1 in text).
NC	Number of cases considered.
NDATA	Data input code. If NDATA=1, new cards are read in for that case, if NDATA=0, all or part of the data from the previous example are used for the new problem.
NIT	Iteration number in least-squares analysis.
NOB	Number of observations (cannot exceed 90 with currently dimensioned arrays).
NP	Number of variable parameters to be fitted to the data.
NPRNT	Output print code. If NPRNT=1, the input c(x,t) array is printed out; NPRNT=0 suppresses this printing.
NREDU	Data input code. If NREDU=0, input concentrations and times are not reduced. If NREDU=1, only the input concentrations are reduced. If NREDU=2, input concentrations and times are both assumed to be dimensionless.
NSKIP	Input file execution code. If NSKIP=0, the problem is executed; if NSKIP≠0, the problem is read in but not executed.
OMEGA	Dimensionless coefficient $\omega$ for Models 3 and 4 [ $L^0$ ].
R	Retardation factor [ $L^0$ ].
RX0	Zero-order rate constant, $\gamma$ (positive for production, negative for decay) [ $ML^{-3}T^{-1}$ ].
RXOR	Value of $\gamma/R$ [ $ML^{-3}T^{-1}$ ].
RX1	First-order decay constant, $\mu$ [ $T^{-1}$ ].
RX1R	Value of $\mu/R$ [ $T^{-1}$ ].
SDLN	Value of $\sigma_{1n}$ in Models 5-8.

SOLOAD	Value of $M_0/\theta$ in Models 7 and 8 [ $ML^{-2}$ ].
STOPCR	Stop criterion. The curve-fitting process stops when the relative change in the ratio of all coefficients becomes less than STOPCR (currently set to .0005 in the program).
T(I)	Time corresponding to the I-th observed concentration value [T].
TITLE	Vector containing information of title cards.
TO	Pulse duration for Models 1-6.
V	Pore water velocity, $v$ [ $LT^{-1}$ ].
VLNM	Value of $\mu_{1n}$ in Models 5-8.
VR	Value of $v/R$ [ $LT^{-1}$ ].
X(I)	Distance from source corresponding to the I-th observed concentration value [L].
XL	Maximum value L of X(I) array [L].
Z	Dimensionless distance, $x/L$ [ $L^0$ ].

---

11. APPENDIX B. Data input instructions.

Card	Columns	Format	Variable	Comments
1	1-10	I10	NC	Number of cases considered. The remaining cards are read in for each case. If NDATA=0 on card 2, data cards 9, etc. are not needed for that particular case.
2	1-10	I10	MODE	Model number. See Table 1 of text for explanation.
2	11-20	I10	NDATA	New data input code. See Appendix A for explanation.
2	21-30	I10	NREDU	Reduced data input code. See Appendix A for explanation.
2	31-40	I10	MIT	Maximum number of iterations. See Appendix A for explanation.
2	41-50	I10	NOB	Number of observations. See Appendix A for explanation.
2	51-60	I10	NSKIP	Input execution code. See Appendix A for explanation.
2	61-70	I10	NPRNT	Output print code. See Appendix A for explanation.
3	1-80	20A4	TITLE	Information card 1.
4	1-80	20A4	TITLE	Information card 2.
5	5-10	A4,A2	BI(1)	Name of each coefficient B(I). Coefficient 7 applies only for Models 5-8. See Appendix A and Table 1 of text for additional explanation.
5	15-20	A4,A2	BI(2)	
5	25-30	A4,A2	BI(3)	
5	35-40	A4,A2	BI(4)	
5	45-50	A4,A2	BI(5)	
5	55-60	A4,A2	BI(6)	
5	65-70	A4,A2	BI(7)	



6	1-10	F10.0	B(1)	Initial value for each coefficient. See Table 1 of text.
6	11-20	F10.0	B(2)	
6	21-30	F10.0	B(3)	
6	31-40	F10.0	B(4)	
6	41-50	F10.0	B(5)	
6	51-60	F10.0	B(6)	
6	61-70	F10.0	B(7)	
7	1-10	I10	INDEX(1)	Index for each coefficient. See Appendix A and text for explanation.
7	11-20	I10	INDEX(2)	
7	21-30	I10	INDEX(3)	
7	31-40	I10	INDEX(4)	
7	41-50	I10	INDEX(5)	
7	51-60	I10	INDEX(6)	
7	61-70	I10	INDEX(7)	
8	1-10	F10.0	CI	Initial concentration.
8	11-20	F10.0	CO	Influent concentration.
9	1-10	F10.0	C(I)	Value of the Ith observed concentration (blank for MIT=0).
9	11-20	F10.0	X(I)	Distance from source for observation I.
9	21-30	F10.0	T(I)	Elapsed time for observation I.

Card 9 is repeated NOB times.

---



12. APPENDIX C.

Input files for selected examples  
discussed in text.

1 2 3 4 5 6 7  
 12345678901234567890123456789012345678901234567890

7 1 0 0 22 0 0  
 EX.1A: HYPOTHETICAL CASE OF EQUILIBRIUM ADSORPTION  
 (UNITS: CENTIMETERS, DAYS, MICROGRAMS)  
 V..... D..... R..... TO..... RX1... RX0...  
 25.0 100.0 2.5 5.0 0.25 0.5  
 0 0 0 0 0 0  
 0.0 100.0  
 0.0 5.0  
 10.0 5.0  
 20.0 5.0  
 30.0 5.0  
 40.0 5.0  
 50.0 5.0  
 60.0 5.0  
 70.0 5.0  
 80.0 5.0  
 90.0 5.0  
 25.0 5.0  
 0.0 10.0  
 10.0 10.0  
 20.0 10.0  
 30.0 10.0  
 40.0 10.0  
 50.0 10.0  
 60.0 10.0  
 70.0 10.0  
 80.0 10.0  
 90.0 10.0  
 100.0 10.0  
 1 1 0 30 11 0 0

EX.1C: HYPOTHETICAL CASE OF EQUILIBRIUM ADSORPTION  
 FIT TO EX.1A DATA (UNITS: CENTIMETERS, DAYS, MICROGRAMS)  
 V..... D..... R..... TO..... RX1... RX0...  
 25.0 1.0 1.0 5.0 1.0 1.0  
 0 1 1 0 1 1  
 0.0 100.0  
 96.23 0.0 5.0  
 86.84 10.0 5.0  
 76.73 20.0 5.0  
 64.66 30.0 5.0  
 50.24 40.0 5.0  
 34.71 50.0 5.0  
 20.75 60.0 5.0  
 10.63 70.0 5.0  
 4.75 80.0 5.0  
 2.08 90.0 5.0  
 1.13 100.0 5.0  
 4 1 0 30 30 0 0

EX.2B: HYPOTHETICAL CASE OF NONEQUILIBRIUM TRANSPORT  
 (UNITS: CENTIMETERS, DAYS, MICROGRAMS)

V.....	D.....	R.....	PULSE.	BETA..	OMEGA.
10.0	1.0	1.0	1.0	0.5	1.0
0	1	1	1	1	1
0.0	500.0				
0.0	20.0	0.2			
0.0	20.0	0.4			
0.00	20.0	0.6			
0.04	20.0	0.8			
1.36	20.0	1.0			
49.95	20.0	1.5			
134.25	20.0	2.0			
206.49	20.0	3.0			
198.76	20.0	4.0			
105.25	20.0	5.0			
89.12	20.0	6.0			
77.63	20.0	7.0			
67.25	20.0	8.0			
49.70	20.0	10.0			
36.12	20.0	12.0			
0.0	40.0	0.2			
0.0	40.0	0.4			
0.0	40.0	0.6			
0.0	40.0	0.8			
0.0	40.0	1.0			
0.00	40.0	1.5			
0.01	40.0	2.0			
11.64	40.0	3.0			
56.57	40.0	4.0			
90.53	40.0	5.0			
90.09	40.0	6.0			
80.44	40.0	7.0			
78.75	40.0	8.0			
73.38	40.0	10.0			
65.08	40.0	12.0			
2	1	2	30	14	0

EX.3A: BROMIDE EFFLUENT FROM 'WORMHOLE' COLUMN  
(UNITS: METERS, DAYS, ARBITRARY MASS UNITS)

V.....	D.....	R.....	PULSE.	RX1...	RX0...
107.0	10.0	1.0	0.65	0.0	0.0
0	1	0	0	0	0
0.0	1.0				
0.8060	0.19	0.030			
0.9420	0.19	0.147			
0.9650	0.19	0.272			
0.9600	0.19	0.410			
0.9800	0.19	0.558			
0.8820	0.19	0.613			
0.1850	0.19	0.662			
0.0226	0.19	0.798			
0.0101	0.19	0.949			
0.0077	0.19	1.093			
0.0064	0.19	1.244			
0.0057	0.19	1.409			
0.0054	0.19	1.578			
0.0050	0.19	1.751			

4	1	2	30	30	0	0
EX.4A: BORON EFFLUENT FROM GLENDALE CLAY LOAM						
(UNITS: CENTIMETERS, DAYS, MICROGRAMS)						
V.....	D.....	R.....	PULSE.	BETA..	OMEGA.	
38.5	2.0	10.0	6.49	0.2	0.2	
0	1	1	0	1	1	
0.0	20.0					
0.015	30.0	1.80				
0.075	30.0	1.95				
0.170	30.0	2.10				
0.265	30.0	2.25				
0.340	30.0	2.40				
0.430	30.0	2.60				
0.535	30.0	2.85				
0.620	30.0	3.15				
0.687	30.0	3.50				
0.738	30.0	4.00				
0.777	30.0	4.60				
0.819	30.0	5.30				
0.852	30.0	6.00				
0.880	30.0	6.70				
0.882	30.0	7.30				
0.852	30.0	7.75				
0.776	30.0	8.00				
0.699	30.0	8.25				
0.621	30.0	8.55				
0.527	30.0	8.90				
0.433	30.0	9.30				
0.357	30.0	9.80				
0.269	30.0	10.50				
0.186	30.0	11.50				
0.133	30.0	12.70				
0.090	30.0	14.00				
0.054	30.0	15.50				
0.040	30.0	17.00				
0.029	30.0	18.50				
0.025	30.0	20.00				
7	1	0	30	30	0	0
EX.5B: HYPOTHETICAL REGIONAL TRANSPORT PROBLEM						
FIT TO EX.5A (UNITS: CENTIMETERS, DAYS, MICROGRAMS)						
<V>...	DISP..	R.....	SOLOAD	RX1...	RX0...	SDLN..
10.0	1.0	1.0	20000.0	0.0	0.0	1.0
1	1	1	0	0	0	1
0.0	1000.0					
68.14	10.00	1.00				
15.26	25.00	1.00				
2.84	50.00	1.00				
0.87	75.00	1.00				
0.35	100.00	1.00				
0.09	150.00	1.00				
0.03	200.00	1.00				
0.01	300.00	1.00				
0.00	400.00	1.00				
0.00	500.00	1.00				
94.94	10.00	5.00				

41.89	25.00	5.00
14.91	50.00	5.00
6.85	75.00	5.00
3.67	100.00	5.00
1.39	150.00	5.00
0.65	200.00	5.00
0.21	300.00	5.00
0.09	400.00	5.00
0.04	500.00	5.00
79.38	10.00	10.00
45.49	25.00	10.00
20.94	50.00	10.00
11.34	75.00	10.00
6.86	100.00	10.00
3.07	150.00	10.00
1.63	200.00	10.00
0.62	300.00	10.00
0.29	400.00	10.00
0.16	500.00	10.00

5 1 1 30 10 0 0  
 EX.6A: FIELD-SCALE BROMIDE MOVEMENT (JURY ET AL.,1982)  
 (UNITS: MILIMETERS, DAYS, ARBITRARY MASS--REDUCED C)

<V>*. .	DISP..	R.....	PULSE.	RX1...	RX0...	SDLN..
25.0	100.0	1.0	1.69	0.0	0.0	1.0
1	1	0	0	0	0	1
0.0	1.0					
0.062	300.0	5.0				
0.076	300.0	10.0				
0.059	300.0	15.0				
0.038	300.0	20.0				
0.014	300.0	25.0				
0.007	300.0	30.0				
0.004	300.0	35.0				
0.003	300.0	40.0				
0.001	300.0	45.0				
0.000	300.0	50.0				





### 13. APPENDIX D

Output files for selected examples  
discussed in text.

```

*****
*
*       ONE-DIMENSIONAL CONVECTION-DISPERSION EQ. SOLUTION
*
*       DETERMINISTIC LINEAR EQUILIBRIUM ADSORPTION FOR PULSE INJECTION
*       WITH FIRST- AND ZERO-ORDER PRODUCTION AND DECAY
*       SOLUTION FOR RESIDENT CONCENTRATIONS
*
*       EX.1A: HYPOTHETICAL CASE OF EQUILIBRIUM ADSORPTION
*       (UNITS: CENTIMETERS, DAYS, MICROGRAMS)
*
*****

```

# ===== INITIAL VALUES OF COEFFICIENTS =====

NAME	INITIAL VALUE
V.....	25.0000
D.....	100.0000
R.....	2.5000
T0.....	5.0000
RX1.....	0.2500
RX0.....	0.5000
C1.....	0.0
C0.....	100.0000

# -----RESULTS FOR INITIAL COEFFICIENT VALUES-----

NO	DISTANCE	TIME	CONCENTRATION
1	0.0	5.0000	96.2355
2	10.0000	5.0000	86.8428
3	20.0000	5.0000	76.7266
4	30.0000	5.0000	64.6646
5	40.0000	5.0000	50.2391
6	50.0000	5.0000	34.7099
7	60.0000	5.0000	20.7448
8	70.0000	5.0000	10.6034
9	80.0000	5.0000	4.7493
10	90.0000	5.0000	2.0823
11	25.0000	5.0000	70.9998
12	0.0	10.0000	0.2022
13	10.0000	10.0000	1.0973
14	20.0000	10.0000	3.4644
15	30.0000	10.0000	8.3961
16	40.0000	10.0000	16.1408
17	50.0000	10.0000	25.2015
18	60.0000	10.0000	32.6311
19	70.0000	10.0000	35.9173
20	80.0000	10.0000	34.4879
21	90.0000	10.0000	29.5869
22	100.0000	10.0000	23.1039

```

*****
ONE-DIMENSIONAL CONVECTION-DISPERSION EQ. SOLUTION
NON-LINEAR LEAST-SQUARES ANALYSIS
*****
DETERMINISTIC LINEAR EQUILIBRIUM ADSORPTION FOR PULSE INJECTION
WITH FIRST- AND ZERO-ORDER PRODUCTION AND DECAY
SOLUTION FOR RESIDENT CONCENTRATIONS
*****
EX.1C: HYPOTHETICAL CASE OF EQUILIBRIUM ADSORPTION
FIT TO EX.1A DATA (UNITS: CENTIMETERS, DAYS, MICROGRAMS)
*****

```

# INITIAL VALUES OF COEFFICIENTS

```

=====
NAME          INITIAL VALUE
.....
..... 25.0000
..... 1.0000
..... 1.0000
0..... 5.0000
X1..... 1.0000
X0..... 1.0000
I..... 0.0
0..... 100.0000

```

TERATION	SSQ	D.....	R.....	RX1...	RX0...
0	3917.13832	1.00000	1.00000	1.00000	1.00000
1	3912.71790	1.00000	1.49210	1.00000	1.00000
2	458.36066	9.82327	1.94834	0.39060	2.85959
3	90.39628	25.50612	2.04880	0.41171	1.09147
4	16.03210	59.46360	2.24656	0.34172	0.54057
5	0.81370	89.70647	2.43398	0.27487	0.48151
6	0.00828	99.49115	2.49552	0.25169	0.49078
7	0.00039	100.25669	2.50052	0.24964	0.49666
8	0.00038	100.27995	2.50062	0.24960	0.49705
9	0.00038	100.28036	2.50062	0.24960	0.49706

# ORRELATION MATRIX

```

=====
      1      2      3      4
1  1.0000
2  0.8601  1.0000
3 -0.9592 -0.8974  1.0000
4 -0.6091 -0.2490  0.6051  1.0000

```

SQARE FOR REGRESSION OF OBSERVED VS PREDICTED=0.99999997

# NON-LINEAR LEAST SQUARES ANALYSIS, FINAL RESULTS

=====

VARIABLE	NAME	VALUE	S.E. COEFF.	T-VALUE	95% CONFIDENCE LIMITS	
					LOWER	UPPER
1	D.....	100.28036	0.15469	648.29	99.91460	100.64612
2	R.....	2.50062	0.00066	3768.48	2.49905	2.50219
3	RX1...	0.24960	0.00030	840.28	0.24890	0.25031
4	RX0...	0.49706	0.00413	120.34	0.48730	0.50683

## -----ORDERED BY COMPUTER INPUT-----

NO	DISTANCE	TIME	CONCENTRATION		RESI-DUAL
			OBS	FITTED	
1	0.0	5.0000	96.2300	96.2293	0.0007
2	10.0000	5.0000	86.8400	86.8437	-0.0037
3	20.0000	5.0000	76.7300	76.7272	0.0028
4	30.0000	5.0000	64.6600	64.6615	-0.0015
5	40.0000	5.0000	50.2400	50.2360	0.0040
6	50.0000	5.0000	34.7100	34.7133	-0.0033
7	60.0000	5.0000	20.7500	20.7558	-0.0058
8	70.0000	5.0000	10.6300	10.6163	0.0137
9	80.0000	5.0000	4.7500	4.7578	-0.0078
10	90.0000	5.0000	2.0800	2.0848	-0.0048
11	100.0000	5.0000	1.1300	1.1250	0.0050

## -----ORDERED BY RESIDUAL-----

NO	DISTANCE	TIME	CONCENTRATION		RESI-DUAL
			OBS	FITTED	
8	70.0000	5.0000	10.6300	10.6163	0.0137
11	100.0000	5.0000	1.1300	1.1250	0.0050
5	40.0000	5.0000	50.2400	50.2360	0.0040
3	20.0000	5.0000	76.7300	76.7272	0.0028
1	0.0	5.0000	96.2300	96.2293	0.0007
4	30.0000	5.0000	64.6600	64.6615	-0.0015
6	50.0000	5.0000	34.7100	34.7133	-0.0033
2	10.0000	5.0000	86.8400	86.8437	-0.0037
10	90.0000	5.0000	2.0800	2.0848	-0.0048
7	60.0000	5.0000	20.7500	20.7558	-0.0058
9	80.0000	5.0000	4.7500	4.7578	-0.0078

```

*****
*
*       ONE-DIMENSIONAL CONVECTION-DISPERSION EQ. SOLUTION
*       NON-LINEAR LEAST-SQUARES ANALYSIS
*
*       DETERMINISTIC TWO-SITE/TWO-REGION NONEQUILIBRIUM MODEL FOR
*       PULSE-TYPE INJECTION WITH NO PRODUCTION OR DECAY
*       SOLUTION FOR FLUX CONCENTRATIONS
*
*       EX.2B: HYPOTHETICAL CASE OF NONEQUILIBRIUM TRANSPORT
*       (UNITS: CENTIMETERS, DAYS, MICROGRAMS)
*
*****

```

# INITIAL VALUES OF COEFFICIENTS

=====

NAME	INITIAL VALUE
V.....	10.0000
D.....	1.0000
R.....	1.0000
PULSE.....	1.0000
BETA.....	0.5000
OMEGA.....	1.0000
CI.....	0.0
CO.....	500.0000

ITERATION	SSQ	D.....	R.....	PULSE.	BETA..	OMEGA.
0	246022.36623	1.00000	1.00000	1.00000	0.50000	1.00000
1	128996.36620	0.81180	1.63115	1.03430	0.16113	3.35233
2	58793.65999	8.05817	2.45991	1.71952	0.00106	6.80566
3	30511.23505	6.56908	2.33221	2.21464	0.21050	7.77231
4	22294.31832	20.03290	2.42207	2.03889	0.26845	8.38396
5	10681.56125	19.39181	2.32613	2.15505	0.27720	5.79992
6	6515.38259	11.36916	2.39445	2.15037	0.45088	1.61625
7	4065.76046	7.47247	3.05962	2.50670	0.26818	2.42405
8	81.72203	7.03156	3.02647	2.51254	0.30288	2.49950
9	3.40572	4.65404	2.99762	2.49972	0.29603	2.51777
10	0.58235	4.95646	2.99820	2.49870	0.29739	2.51109
11	0.57551	4.97379	2.99812	2.49853	0.29749	2.51090
12	0.57547	4.97406	2.99811	2.49853	0.29749	2.51088

## CORRELATION MATRIX

	1	2	3	4	5
1	1.0000				
2	0.3155	1.0000			
3	0.2718	0.7586	1.0000		
4	0.4293	-0.6260	-0.6191	1.0000	
5	-0.6169	-0.2879	-0.0743	-0.3728	1.0000

RSQUARE FOR REGRESSION OF OBSERVED VS PREDICTED=0.99999404

## NON-LINEAR LEAST SQUARES ANALYSIS, FINAL RESULTS

VARIABLE	NAME	VALUE	S.E. COEFF.	T-VALUE	95% CONFIDENCE LIMITS	
					LOWER	UPPER
1	D.....	4.97406	0.03742	132.92	4.89698	5.05113
2	R.....	2.99811	0.00270	1111.47	2.99256	3.00367
3	PULSE.	2.49853	0.00138	1806.39	2.49568	2.50138
4	BETA..	0.29749	0.00034	884.54	0.29680	0.29819
5	OMEGA.	2.51088	0.00223	1124.95	2.50628	2.51548

-----ORDERED BY COMPUTER INPUT-----					
NO	DISTANCE	TIME	CONCENTRATION		RESI-DUAL
			OBS	FITTED	
1	20.0000	0.2000	0.0	0.0	0.0
2	20.0000	0.4000	0.0	0.0	0.0
3	20.0000	0.6000	0.0	0.0001	-0.0001
4	20.0000	0.8000	0.0400	0.0441	-0.0041
5	20.0000	1.0000	1.3600	1.4077	-0.0477
6	20.0000	1.5000	49.9500	49.7093	0.2407
7	20.0000	2.0000	134.2500	134.4251	-0.1751
8	20.0000	3.0000	206.4900	206.3964	0.0936
9	20.0000	4.0000	198.7600	198.7150	0.0450
10	20.0000	5.0000	105.2500	105.7654	-0.5154
11	20.0000	6.0000	89.1200	89.0365	0.0835
12	20.0000	7.0000	77.6300	77.5696	0.0604
13	20.0000	8.0000	67.2500	67.2064	0.0436
14	20.0000	10.0000	49.7000	49.6825	0.0175
15	20.0000	12.0000	36.1200	36.1054	0.0146
16	40.0000	0.2000	0.0	0.0	0.0
17	40.0000	0.4000	0.0	0.0	0.0
18	40.0000	0.6000	0.0	0.0	0.0
19	40.0000	0.8000	0.0	0.0	0.0

20	40.0000	1.0000	0.0	0.0	0.0
21	40.0000	1.5000	0.0	0.0000	-0.0000
22	40.0000	2.0000	0.0100	0.0164	-0.0064
23	40.0000	3.0000	11.6400	11.8712	-0.2312
24	40.0000	4.0000	56.5700	56.4797	0.0903
25	40.0000	5.0000	90.5300	90.7646	-0.2346
26	40.0000	6.0000	90.0900	89.9546	0.1354
27	40.0000	7.0000	80.4400	80.4107	0.0293
28	40.0000	8.0000	78.7500	78.5777	0.1723
29	40.0000	10.0000	73.3800	73.2438	0.1362
30	40.0000	12.0000	65.0800	64.9753	0.1047

-----ORDERED BY RESIDUAL-----

NO	DISTANCE	TIME	CONCENTRATION		RESI- DUAL
			OBS	FITTED	
6	20.0000	1.5000	49.9500	49.7093	0.2407
28	40.0000	8.0000	78.7500	78.5777	0.1723
29	40.0000	10.0000	73.3800	73.2438	0.1362
26	40.0000	6.0000	90.0900	89.9546	0.1354
30	40.0000	12.0000	65.0800	64.9753	0.1047
8	20.0000	3.0000	206.4900	206.3964	0.0936
24	40.0000	4.0000	56.5700	56.4797	0.0903
11	20.0000	6.0000	89.1200	89.0365	0.0835
12	20.0000	7.0000	77.6300	77.5696	0.0604
9	20.0000	4.0000	198.7600	198.7150	0.0450
13	20.0000	8.0000	67.2500	67.2064	0.0436
27	40.0000	7.0000	80.4400	80.4107	0.0293
14	20.0000	10.0000	49.7000	49.6825	0.0175
15	20.0000	12.0000	36.1200	36.1054	0.0146
1	20.0000	0.2000	0.0	0.0	0.0
2	20.0000	0.4000	0.0	0.0	0.0
16	40.0000	0.2000	0.0	0.0	0.0
17	40.0000	0.4000	0.0	0.0	0.0
18	40.0000	0.6000	0.0	0.0	0.0
19	40.0000	0.8000	0.0	0.0	0.0
20	40.0000	1.0000	0.0	0.0	0.0
21	40.0000	1.5000	0.0	0.0000	-0.0000
3	20.0000	0.6000	0.0	0.0001	-0.0001
4	20.0000	0.8000	0.0400	0.0441	-0.0041
22	40.0000	2.0000	0.0100	0.0164	-0.0064
5	20.0000	1.0000	1.3600	1.4077	-0.0477
7	20.0000	2.0000	134.2500	134.4251	-0.1751
23	40.0000	3.0000	11.6400	11.8712	-0.2312
25	40.0000	5.0000	90.5300	90.7646	-0.2346
10	20.0000	5.0000	105.2500	105.7654	-0.5154

```

*****
*
*          ONE-DIMENSIONAL CONVECTION-DISPERSION EQ. SOLUTION
*          NON-LINEAR LEAST-SQUARES ANALYSIS
*
*          DETERMINISTIC LINEAR EQUILIBRIUM ADSORPTION FOR PULSE INJECTION
*          WITH FIRST- AND ZERO-ORDER PRODUCTION AND DECAY
*          SOLUTION FOR FLUX CONCENTRATIONS
*          REDUCED CONCENTRATION AND TIME DATA
*
*          EX.3A: BROMIDE EFFLUENT FROM 'WORMHOLE' COLUMN
*          (UNITS: METERS, DAYS, ARBITRARY MASS UNITS)
*
*****

```

# INITIAL VALUES OF COEFFICIENTS

```

=====
NAME          INITIAL VALUE
V.....    107.0000
D.....    10.0000
R.....    1.0000
PULSE.....  0.6500
RX1.....   0.0
RX0.....   0.0
CI.....   0.0
CO.....   1.0000

```

```

ITERATION    SSQ          D.....
0            4.10659      10.00000
1            2.46778      37.23245
2            1.41558      123.82341
3            0.64858      400.18590
4            0.19115      1229.76741
5            0.04893      2937.47280
6            0.01575      5485.46972
7            0.01076      7779.78289
8            0.01048      8660.86768
9            0.01048      8744.04483
10           0.01048      8745.07988

```

RSQUARE FOR REGRESSION OF OBSERVED VS PREDICTED=0.99683340



# NON-LINEAR LEAST SQUARES ANALYSIS, FINAL RESULTS

VARIABLE	NAME	VALUE	S.E. COEFF.	95% CONFIDENCE LIMITS	
1	D.....	8745.07988	1786.53980	LOWER 4885.38692	UPPER 12604.77285

## -----ORDERED BY COMPUTER INPUT-----

NO	DISTANCE	TIME	CONCENTRATION		RESI- DUAL
			OBS	FITTED	
1	0.1900	0.0300	0.8060	0.8449	-0.0389
2	0.1900	0.1470	0.9420	0.9302	0.0118
3	0.1900	0.2720	0.9650	0.9490	0.0160
4	0.1900	0.4100	0.9600	0.9586	0.0014
5	0.1900	0.5580	0.9800	0.9647	0.0153
6	0.1900	0.6130	0.8820	0.9664	-0.0844
7	0.1900	0.6620	0.1850	0.2112	-0.0262
8	0.1900	0.7980	0.0226	0.0402	-0.0176
9	0.1900	0.9490	0.0101	0.0218	-0.0117
10	0.1900	1.0930	0.0077	0.0148	-0.0071
11	0.1900	1.2440	0.0064	0.0109	-0.0045
12	0.1900	1.4090	0.0057	0.0083	-0.0026
13	0.1900	1.5780	0.0054	0.0066	-0.0012
14	0.1900	1.7510	0.0050	0.0054	-0.0004

## -----ORDERED BY RESIDUAL-----

NO	DISTANCE	TIME	CONCENTRATION		RESI- DUAL
			OBS	FITTED	
3	0.1900	0.2720	0.9650	0.9490	0.0160
5	0.1900	0.5580	0.9800	0.9647	0.0153
2	0.1900	0.1470	0.9420	0.9302	0.0118
4	0.1900	0.4100	0.9600	0.9586	0.0014
14	0.1900	1.7510	0.0050	0.0054	-0.0004
13	0.1900	1.5780	0.0054	0.0066	-0.0012
12	0.1900	1.4090	0.0057	0.0083	-0.0026
11	0.1900	1.2440	0.0064	0.0109	-0.0045
10	0.1900	1.0930	0.0077	0.0148	-0.0071
9	0.1900	0.9490	0.0101	0.0218	-0.0117
8	0.1900	0.7980	0.0226	0.0402	-0.0176
7	0.1900	0.6620	0.1850	0.2112	-0.0262
1	0.1900	0.0300	0.8060	0.8449	-0.0389
6	0.1900	0.6130	0.8820	0.9664	-0.0844

```

*****
*
*       ONE-DIMENSIONAL CONVECTION-DISPERSION EQ. SOLUTION
*       NON-LINEAR LEAST-SQUARES ANALYSIS
*
*       DETERMINISTIC TWO-SITE/TWO-REGION NONEQUILIBRIUM MODEL FOR
*       PULSE-TYPE INJECTION WITH NO PRODUCTION OR DECAY
*       SOLUTION FOR FLUX CONCENTRATIONS
*       REDUCED CONCENTRATION AND TIME DATA
*
*       EX.4A: BORON EFFLUENT FROM GLENDALE CLAY LOAM
*       (UNITS: CENTIMETERS, DAYS, MICROGRAMS)
*
*****

```

# INITIAL VALUES OF COEFFICIENTS

```

=====
NAME          INITIAL VALUE
V.....     38.5000
D.....       2.0000
R.....     10.0000
PULSE.....    6.4900
BETA.....     0.2000
OMEGA.....    0.2000
CI.....       0.0
CO.....     20.0000

```

ITERATION	SSQ	D.....	R.....	BETA..	OMEGA.
0	1.86497	2.00000	10.00000	0.20000	0.20000
1	0.89540	5.55891	8.09839	0.24822	0.44224
2	0.41722	15.45646	7.03675	0.30592	0.51867
3	0.20896	33.01378	5.60759	0.40860	0.44036
4	0.19442	48.25238	3.91843	0.57399	0.41148
5	0.05591	60.59524	4.26294	0.60434	0.40417
6	0.05290	48.80769	4.28093	0.59715	0.41627

NO FURTHER DECREASE IN SSQ OBTAINED AFTER 50 TRIALS

# CORRELATION MATRIX

	1	2	3	4
1	1.0000			
2	0.0834	1.0000		
3	0.5889	-0.6561	1.0000	
4	-0.7262	-0.3436	-0.4080	1.0000

RSQUARE FOR REGRESSION OF OBSERVED VS PREDICTED=0.98139377

## NON-LINEAR LEAST SQUARES ANALYSIS, FINAL RESULTS

VARIABLE	NAME	VALUE	S.E. COEFF.	T-VALUE	95% CONFIDENCE LIMITS	
					LOWER	UPPER
1	D.....	48.80769	13.39505	3.64	21.27210	76.34327
2	R.....	4.28093	0.22611	18.93	3.81611	4.74574
3	BETA..	0.59715	0.03308	18.05	0.52914	0.66516
4	OMEGA.	0.41627	0.09013	4.62	0.23099	0.60156

-----ORDERED BY COMPUTER INPUT-----					
NO	DISTANCE	TIME	CONCENTRATION		RESI-DUAL
			OBS	FITTED	
1	30.0000	1.8000	0.0150	0.1035	-0.0885
2	30.0000	1.9500	0.0750	0.1558	-0.0808
3	30.0000	2.1000	0.1700	0.2152	-0.0452
4	30.0000	2.2500	0.2650	0.2779	-0.0129
5	30.0000	2.4000	0.3400	0.3406	-0.0006
6	30.0000	2.6000	0.4300	0.4195	0.0105
7	30.0000	2.8500	0.5350	0.5053	0.0297
8	30.0000	3.1500	0.6200	0.5862	0.0338
9	30.0000	3.5000	0.6870	0.6536	0.0334
10	30.0000	4.0000	0.7380	0.7115	0.0265
11	30.0000	4.6000	0.7770	0.7574	0.0196
12	30.0000	5.3000	0.8190	0.7927	0.0263
13	30.0000	6.0000	0.8520	0.8198	0.0322
14	30.0000	6.7000	0.8800	0.8427	0.0373
15	30.0000	7.3000	0.8820	0.8599	0.0221
16	30.0000	7.7500	0.8520	0.8644	-0.0124
17	30.0000	8.0000	0.7760	0.8445	-0.0685
18	30.0000	8.2500	0.6990	0.7922	-0.0932
19	30.0000	8.5500	0.6210	0.6911	-0.0701
20	30.0000	8.9000	0.5270	0.5524	-0.0254

21	30.0000	9.3000	0.4330	0.4121	0.0209
22	30.0000	9.8000	0.3570	0.2934	0.0636
23	30.0000	10.5000	0.2690	0.2121	0.0569
24	30.0000	11.5000	0.1860	0.1584	0.0276
25	30.0000	12.7000	0.1330	0.1238	0.0092
26	30.0000	14.0000	0.0900	0.0964	-0.0064
27	30.0000	15.5000	0.0540	0.0723	-0.0183
28	30.0000	17.0000	0.0400	0.0541	-0.0141
29	30.0000	18.5000	0.0290	0.0405	-0.0115
30	30.0000	20.0000	0.0250	0.0303	-0.0053

74

-----ORDERED BY RESIDUAL-----					
NO	DISTANCE	TIME	CONCENTRATION		RESI- DUAL
			OBS	FITTED	
22	30.0000	9.8000	0.3570	0.2934	0.0636
23	30.0000	10.5000	0.2690	0.2121	0.0569
14	30.0000	6.7000	0.8800	0.8427	0.0373
8	30.0000	3.1500	0.6200	0.5862	0.0338
9	30.0000	3.5000	0.6870	0.6536	0.0334
13	30.0000	6.0000	0.8520	0.8198	0.0322
7	30.0000	2.8500	0.5350	0.5053	0.0297
24	30.0000	11.5000	0.1860	0.1584	0.0276
10	30.0000	4.0000	0.7380	0.7115	0.0265
12	30.0000	5.3000	0.8190	0.7927	0.0263
15	30.0000	7.3000	0.8820	0.8599	0.0221
21	30.0000	9.3000	0.4330	0.4121	0.0209
11	30.0000	4.6000	0.7770	0.7574	0.0196
6	30.0000	2.6000	0.4300	0.4195	0.0105
25	30.0000	12.7000	0.1330	0.1238	0.0092
5	30.0000	2.4000	0.3400	0.3406	-0.0006
30	30.0000	20.0000	0.0250	0.0303	-0.0053
26	30.0000	14.0000	0.0900	0.0964	-0.0064
29	30.0000	18.5000	0.0290	0.0405	-0.0115
16	30.0000	7.7500	0.8520	0.8644	-0.0124
4	30.0000	2.2500	0.2650	0.2779	-0.0129
28	30.0000	17.0000	0.0400	0.0541	-0.0141
27	30.0000	15.5000	0.0540	0.0723	-0.0183
20	30.0000	8.9000	0.5270	0.5524	-0.0254
3	30.0000	2.1000	0.1700	0.2152	-0.0452
17	30.0000	8.0000	0.7760	0.8445	-0.0685
19	30.0000	8.5500	0.6210	0.6911	-0.0701
2	30.0000	1.9500	0.0750	0.1558	-0.0808
1	30.0000	1.8000	0.0150	0.1035	-0.0885
18	30.0000	8.2500	0.6990	0.7922	-0.0932

```

*****
*
*       ONE-DIMENSIONAL CONVECTION-DISPERSION EQ. SOLUTION
*       NON-LINEAR LEAST-SQUARES ANALYSIS
*
*       STOCHASTIC TRANSPORT MODEL WITH EQUILIBRIUM ADSORPTION
*       ZERO-ORDER PRODUCTION AND FIRST-ORDER DECAY FOR UNIFORM SOLUTE LOAD
*       SOLUTION FOR RESIDENT CONCENTRATIONS
*
*       EX.5B: HYPOTHETICAL REGIONAL TRANSPORT PROBLEM
*       FIT TO EX.5A (UNITS: CENTIMETERS, DAYS, MICROGRAMS)
*
*****

```

# INITIAL VALUES OF COEFFICIENTS

```

=====
NAME          INITIAL VALUE
<V>.....    10.0000
DISP.....    1.0000
R.....       1.0000
SOLOAD.... 20000.0000
RX1.....      0.0
RX0.....      0.0
SDLN.....     1.0000
CI.....       0.0
CO.....     1000.0000

```

ITERATION	SSQ	<V>...	DISP..	R.....	SDLN..
0	212138.759151	10.00000	1.00000	1.00000	1.00000
1	41671.379895	18.41775	0.18538	2.12304	0.99829
2	5685.699634	27.95026	0.47473	3.49626	1.12868
3	290.163378	36.19773	0.71330	4.58340	1.29284
4	14.092663	35.68894	1.91076	4.90676	1.39359
5	10.146888	32.36309	3.55284	4.95535	1.40809
6	9.219075	27.73219	6.60219	5.00988	1.40006
7	0.623445	25.04830	9.65937	5.00271	1.37347
8	0.002403	25.00682	10.01930	4.99709	1.37003
9	0.002053	24.99640	10.03234	4.99673	1.36991
10	0.002045	25.00137	10.02680	4.99668	1.36996
11	0.002045	25.00148	10.02672	4.99668	1.36996

## CORRELATION MATRIX

=====

	1	2	3	4
1	1.0000			
2	-0.9193	1.0000		
3	-0.2474	-0.0124	1.0000	
4	0.6063	-0.3364	-0.8575	1.0000

RSQUARE FOR REGRESSION OF OBSERVED VS PREDICTED=0.99999992

## NON-LINEAR LEAST SQUARES ANALYSIS, FINAL RESULTS

=====

VARIABLE	NAME	VALUE	S.E. COEFF.	T-VALUE	95% CONFIDENCE LIMITS	
					LOWER	UPPER
1	<V>...	25.00148	0.01785	1400.87	24.96480	25.03817
2	DISP..	10.02672	0.01919	522.54	9.98727	10.06616
3	R.....	4.99668	0.00087	5739.96	4.99489	4.99847
4	SDLN..	1.36996	0.00034	3981.22	1.36925	1.37067

## -----ORDERED BY COMPUTER INPUT-----

NO	DISTANCE	TIME	CONCENTRATION		RESI-DUAL
			OBS	FITTED	
1	10.0000	1.0000	68.1400	68.1516	-0.0116
2	25.0000	1.0000	15.2600	15.2561	0.0039
3	50.0000	1.0000	2.8400	2.8408	-0.0008
4	75.0000	1.0000	0.8700	0.8653	0.0047
5	100.0000	1.0000	0.3500	0.3310	0.0190
6	150.0000	1.0000	0.0900	0.0800	0.0100
7	200.0000	1.0000	0.0300	0.0263	0.0037
8	300.0000	1.0000	0.0100	0.0	0.0100
9	400.0000	1.0000	0.0	0.0	0.0
10	500.0000	1.0000	0.0	0.0	0.0
11	10.0000	5.0000	94.9400	94.9317	0.0083
12	25.0000	5.0000	41.8900	41.8875	0.0025
13	50.0000	5.0000	14.9100	14.9128	-0.0028
14	75.0000	5.0000	6.8500	6.8512	-0.0012
15	100.0000	5.0000	3.6700	3.6538	0.0162
16	150.0000	5.0000	1.3900	1.3846	0.0054
17	200.0000	5.0000	0.6500	0.6486	0.0014
18	300.0000	5.0000	0.2100	0.2016	0.0084
19	400.0000	5.0000	0.0900	0.0841	0.0059

20	500.0000	5.0000	0.0400	0.0410	-0.0010
21	10.0000	10.0000	79.3800	79.3887	-0.0087
22	25.0000	10.0000	45.4900	45.4854	0.0046
23	50.0000	10.0000	20.9400	20.9485	-0.0085
24	75.0000	10.0000	11.3400	11.3201	0.0199
25	100.0000	10.0000	6.8600	6.8567	0.0033
26	150.0000	10.0000	3.0700	3.0740	-0.0040
27	200.0000	10.0000	1.6300	1.6167	0.0133
28	300.0000	10.0000	0.6200	0.6138	0.0062
29	400.0000	10.0000	0.2900	0.2919	-0.0019
30	500.0000	10.0000	0.1600	0.1590	0.0010

-----ORDERED BY RESIDUAL-----					
NO	DISTANCE	TIME	CONCENTRATION		RESI- DUAL
			OBS	FITTED	
24	75.0000	10.0000	11.3400	11.3201	0.0199
5	100.0000	1.0000	0.3500	0.3310	0.0190
15	100.0000	5.0000	3.6700	3.6538	0.0162
27	200.0000	10.0000	1.6300	1.6167	0.0133
8	300.0000	1.0000	0.0100	0.0	0.0100
6	150.0000	1.0000	0.0900	0.0800	0.0100
18	300.0000	5.0000	0.2100	0.2016	0.0084
11	10.0000	5.0000	94.9400	94.9317	0.0083
28	300.0000	10.0000	0.6200	0.6138	0.0062
19	400.0000	5.0000	0.0900	0.0841	0.0059
16	150.0000	5.0000	1.3900	1.3846	0.0054
4	75.0000	1.0000	0.8700	0.8653	0.0047
22	25.0000	10.0000	45.4900	45.4854	0.0046
2	25.0000	1.0000	15.2600	15.2561	0.0039
7	200.0000	1.0000	0.0300	0.0263	0.0037
25	100.0000	10.0000	6.8600	6.8567	0.0033
12	25.0000	5.0000	41.8900	41.8875	0.0025
17	200.0000	5.0000	0.6500	0.6486	0.0014
30	500.0000	10.0000	0.1600	0.1590	0.0010
9	400.0000	1.0000	0.0	0.0	0.0
10	500.0000	1.0000	0.0	0.0	0.0
3	50.0000	1.0000	2.8400	2.8408	-0.0008
20	500.0000	5.0000	0.0400	0.0410	-0.0010
14	75.0000	5.0000	6.8500	6.8512	-0.0012
29	400.0000	10.0000	0.2900	0.2919	-0.0019
13	50.0000	5.0000	14.9100	14.9128	-0.0028
26	150.0000	10.0000	3.0700	3.0740	-0.0040
23	50.0000	10.0000	20.9400	20.9485	-0.0085
21	10.0000	10.0000	79.3800	79.3887	-0.0087
1	10.0000	1.0000	68.1400	68.1516	-0.0116

```

*****
*
*       ONE-DIMENSIONAL CONVECTION-DISPERSION EQ. SOLUTION
*       NON-LINEAR LEAST-SQUARES ANALYSIS
*
*       STOCHASTIC TRANSPORT MODEL WITH EQUILIBRIUM ADSORPTION
*       ZERO-ORDER PRODUCTION AND FIRST-ORDER DECAY FOR UNIFORM PULSE INPUT
*       SOLUTION FOR RESIDENT CONCENTRATIONS
*       REDUCED CONCENTRATION DATA
*
*       EX.6A: FIELD-SCALE BROMIDE MOVEMENT (JURY ET AL.,1982)
*       (UNITS: MILIMETERS, DAYS, ARBITRARY MASS--REDUCED C)
*
*****

```

# INITIAL VALUES OF COEFFICIENTS

=====

NAME	INITIAL VALUE
<V>*.....	25.0000
DISP.....	100.0000
R.....	1.0000
PULSE.....	1.6900
RX1.....	0.0
RX0.....	0.0
SDLN.....	1.0000
CI.....	0.0
CO.....	1.0000

ITERATION	SSQ	<V>*..	DISP..	SDLN..
0	0.001957	25.00000	100.00000	1.00000
1	0.001062	27.41737	38.93184	0.91028
2	0.000620	25.72053	77.91321	0.60558
3	0.000545	28.47699	11.22351	0.76138
4	0.000518	29.69033	9.38482	0.76866
5	0.000518	30.44372	0.38794	0.80010
6	0.000518	30.44455	0.38133	0.80012
7	0.000518	30.45577	0.86023	0.79995
8	0.000518	30.45479	0.98181	0.79984
9	0.000518	30.45601	0.97338	0.79984
10	0.000518	30.45886	1.00424	0.79974
11	0.000518	30.46018	0.96600	0.79977
12	0.000518	30.46087	0.99540	0.79974
13	0.000518	30.46136	0.98188	0.79976
14	0.000518	30.46136	0.98188	0.79976



# CORRELATION MATRIX

=====

	1	2	3
1	1.0000		
2	-0.9956	1.0000	
3	0.9954	-0.9977	1.0000

RSQUARE FOR REGRESSION OF OBSERVED VS PREDICTED=0.98518064

## NON-LINEAR LEAST SQUARES ANALYSIS, FINAL RESULTS

=====

VARIABLE	NAME	VALUE	S.E. COEFF.	T-VALUE	95% CONFIDENCE LIMITS	
					LOWER	UPPER
1	<V>*..	30.46136	21.01538	1.45	-19.22989	80.15261
2	DISP..	0.98188	227.53307	0.00	-537.02429	538.98805
3	SDLN..	0.79976	0.94273	0.85	-1.42936	3.02887

-----ORDERED BY COMPUTER INPUT-----

NO	DISTANCE	TIME	CONCENTRATION		RESI-DUAL
			OBS	FITTED	
1	300.0000	5.0000	0.0620	0.0676	-0.0056
2	300.0000	10.0000	0.0760	0.0812	-0.0052
3	300.0000	15.0000	0.0590	0.0592	-0.0002
4	300.0000	20.0000	0.0380	0.0400	-0.0020
5	300.0000	25.0000	0.0140	0.0268	-0.0128
6	300.0000	30.0000	0.0070	0.0183	-0.0113
7	300.0000	35.0000	0.0040	0.0127	-0.0087
8	300.0000	40.0000	0.0030	0.0090	-0.0060
9	300.0000	45.0000	0.0010	0.0065	-0.0055
10	300.0000	50.0000	0.0	0.0047	-0.0047

```

-----ORDERED BY RESIDUAL-----

```

NO	DISTANCE	TIME	OBS CONCENTRATION	FITTED	RESI- DUAL
3	300.0000	15.0000	0.0590	0.0592	-0.0002
4	300.0000	20.0000	0.0380	0.0400	-0.0020
10	300.0000	50.0000	0.0	0.0047	-0.0047
2	300.0000	10.0000	0.0760	0.0812	-0.0052
9	300.0000	45.0000	0.0010	0.0065	-0.0055
1	300.0000	5.0000	0.0620	0.0676	-0.0056
8	300.0000	40.0000	0.0030	0.0090	-0.0060
7	300.0000	35.0000	0.0040	0.0127	-0.0087
6	300.0000	30.0000	0.0070	0.0183	-0.0113
5	300.0000	25.0000	0.0140	0.0268	-0.0128

14. APPENDIX E.

Listing of program CXTFIT for the analysis  
of transport parameters from measured  
resident or flux concentration data

```

C *****
C *
C * CXTFIT: NON-LINEAR LEAST-SQUARES ANALYSIS OF C(X,T) DATA *
C * FOR ONE-DIMENSIONAL DETERMINISTIC OR STOCHASTIC SOLUTE *
C * TRANSPORT WITH DETECTION IN RESIDENT OR FLUX MODES *
C *
C *****
C
C IMPLICIT REAL*8 (A-H,O-Z)
C DIMENSION C(90),F(90),R(90),DELZ(90,7),B(14),E(7),TH(14),P(7),
1 PHI(7),Q(7),LSORT(90),TB(14),A(7,7),BI(14),TITLE(20),D(7,7)
C COMMON/MODAT1/X(90),T(90),INDEX(7),NOB,NREDU,NVAR
C COMMON/MODAT2/C1,C0,DM1,XL,DM2,DM3,DM4,DM5,DM6,DM7,DM8,DM9,DM10,
1 DM11,DM12,MODE,MODT
C DATA STOPCR/0.0005/
C DATA MAXTRY/50/
C
C ----- READ NUMBER OF CASES -----
C READ(5,1006) NC
C DO 150 NCASE=1,NC
C
C ----- READ INPUT PARAMETERS -----
C READ(5,1006) MODE,NDATA,NREDU,MIT,NOB,NSKIP,NPRNT
C IF(NSKIP.NE.0) GO TO 2
C WRITE(6,1000)
C IF(MIT.NE.0) WRITE(6,1035)
C WRITE(6,1036)
C M=(MODE-1)/2
C IF(M.EQ.0) WRITE(6,1021)
C IF(M.EQ.1) WRITE(6,1022)
C IF(M.EQ.2) WRITE(6,1023)
C IF(M.EQ.3) WRITE(6,1039)
C N=MODE-2*M
C IF(N.EQ.1) WRITE(6,1024)
C IF(N.EQ.2) WRITE(6,1025)
C IF(NREDU.EQ.1) WRITE(6,1037)
C IF(NREDU.EQ.2) WRITE(6,1040)
C WRITE(6,1036)
2 NVAR=6
C IF(MODE.GE.5) NVAR=7
C NU1=NVAR+1
C NU2=2*NVAR
C DO 4 I=1,2
C READ(5,1001) TITLE
4 IF(NSKIP.EQ.0) WRITE(6,1002) TITLE
C IF(NSKIP.EQ.0) WRITE(6,1003)
C
C ----- READ COEFFICIENTS NAMES -----
C READ(5,1004) (BI(I),I=1,NU2)
C
C ----- READ INITIAL ESTIMATES -----
C READ(5,1005) (B(I),I=NU1,NU2)
C
C ----- READ INDICES -----
C READ(5,1006) (INDEX(I),I=1,NVAR)
C IF(NSKIP.NE.0) GO TO 8
C WRITE(6,1007)
C DO 6 I=1,NVAR
C J=2*I-1
6 WRITE(6,1008) BI(J),BI(J+1),B(I+NVAR)
8 CONTINUE

```

```

C
C ----- READ INITIAL AND INLET CONCENTRATIONS -----
READ(5,1005) CI,CO
IF(NSKIP.EQ.0) WRITE(6,1026) CI,CO
IF(NDATA.NE.1) GO TO 11
C
C ----- READ AND WRITE EXPERIMENTAL DATA -----
XL=0.0
DO 10 I=1,NOB
READ(5,1005) C(I),X(I),T(I)
10 XL=DMAX1(XL,X(I))
11 IF(NREDU.EQ.2) INDEX(1)=0
IF(NSKIP.NE.0) GO TO 150
IF(MIT.EQ.0) GO TO 18
IF(NPRNT.NE.1) GO TO 15
WRITE(6,1009)
DO 14 I=1,NOB
14 WRITE(6,1010) I,C(I),X(I),T(I)
C
C -----CHECK FOR INPUT ERROR-----
15 NZC=0
NFC=0
DO 17 I=1,NVAR
IF((B(I+NVAR).EQ.0.0).AND.(I.LE.3)) GO TO 129
IF((B(I+NVAR).EQ.0.0).AND.(INDEX(I).EQ.1)) GO TO 130
IF((I.EQ.4).OR.(MODE.GE.5)) GO TO 17
IF(INDEX(I).EQ.1) NFC=NFC+1
IF(B(I+NVAR).NE.0.0) NZC=NZC+1
17 CONTINUE
IF(MODE.GE.5) GO TO 18
IF(NZC.LE.NFC) GO TO 120
C
C ----- REARRANGE VARIABLE ARRAYS -----
18 NP=0
DO 20 I=NU1,NU2
TB(I)=B(I)
IF(INDEX(I-NVAR).EQ.0) GO TO 20
NP=NP+1
K=2*NP-1
J=2*(I-NVAR)-1
BI(K)=BI(J)
BI(K+1)=BI(J+1)
B(NP)=B(I)
TB(NP)=B(I)
TH(NP)=B(NP)
20 TH(I)=B(I)
C
C -----
GA=0.02
NIT=0
NP2=2*NP
CALL MODEL(TH,F)
IF(MIT.EQ.0) GO TO 140
SSQ=0.
DO 32 I=1,NOB
R(I)=C(I)-F(I)
32 SSQ=SSQ+R(I)*R(I)
WRITE(6,1011) (BI(J),BI(J+1),J=1,NP2,2)
WRITE(6,1012) NIT,SSQ,(B(I),I=1,NP)

```

```

C
C ----- BEGIN ITERATION -----
34 NIT=NIT+1
   NTRIAL=0
   GA=0.1*GA
   DO 38 J=1,NP
     TEMP=TH(J)
     TH(J)=1.01*TH(J)
     Q(J)=0.
     CALL MODEL(TH,DELZ(1,J))
     DO 36 I=1,NOB
       DELZ(I,J)=DELZ(I,J)-F(I)
36   Q(J)=Q(J)+DELZ(I,J)*R(I)
     Q(J)=100.*Q(J)/TH(J)
C
C ----- Q=XT*R (STEEPEST DESCENT) -----
38 TH(J)=TEMP
   DO 44 I=1,NP
     DO 42 J=1,I
       SUM=0.
       DO 40 K=1,NOB
40    SUM=SUM+DELZ(K,I)*DELZ(K,J)
       D(I,J)=10000.*SUM/(TH(I)*TH(J))
42    D(J,I)=D(I,J)
       E(I)=DSQRT(D(I,I))
44    IF(E(I).EQ.0.) E(I)=1.E-30
50    DO 52 I=1,NP
       DO 52 J=1,NP
52    A(I,J)=D(I,J)/(E(I)*E(J))
C
C ----- A IS THE SCALED MOMENT MATRIX -----
   DO 54 I=1,NP
     P(I)=Q(I)/E(I)
     PHI(I)=P(I)
54    A(I,I)=A(I,I)+GA
     CALL MATINV(A,NP,P)
C
C ----- P/E IS THE CORRECTION VECTOR -----
   STEP=1.0
56   DO 58 I=1,NP
58    TB(I)=P(I)*STEP/E(I)+TH(I)
     DO 62 I=1,NP
       IF(TH(I)*TB(I))66,66,62
62   CONTINUE
     SUMB=0.0
     CALL MODEL(TB,F)
     DO 64 I=1,NOB
       R(I)=C(I)-F(I)
64    SUMB=SUMB+R(I)*R(I)
66    SUM1=0.0
     SUM2=0.0
     SUM3=0.0
     DO 68 I=1,NP
       SUM1=SUM1+P(I)*PHI(I)
       SUM2=SUM2+P(I)*P(I)
68    SUM3=SUM3+PHI(I)*PHI(I)
     ARG=SUM1/DSQRT(SUM2*SUM3)
     ARG1=0.0
     IF(NP.GT.1) ARG1=DSQRT(1.-ARG*ARG)
     ANGLE=57.29578*DATAN2(ARG1,ARG)

```

```

C
C      -----
      DO 72 I=1,NP
      IF (TH(I)*TB(I)) 74,74,72
72  CONTINUE
      NTRIAL=NTRIAL+1
      IF (NTRIAL.GT.MAXTRY) GO TO 95
      IF (SUMB/SSQ-1.0) 80,80,74
74  IF (ANGLE-30.0) 76,76,78
76  STEP=0.5*STEP
      GO TO 56
78  GA=10.*GA
      GO TO 50

C
C      ----- PRINT COEFFICIENTS AFTER EACH ITERATION -----
80  CONTINUE
      DO 82 I=1,NP
      TH(I)=TB(I)
      WRITE (6,1012) NIT,SUMB,(TH(I),I=1,NP)
      DO 86 I=1,NP
      IF (DABS(P(I)*STEP/E(I))/(1.0D-20+DABS(TH(I)))-STOPCR) 86,86,94
86  CONTINUE
      GO TO 96
94  SSQ=SUMB
      IF (NIT.LT.MIT) GO TO 34
      IF (NIT.EQ.MIT) WRITE(6,1034) MIT
      GO TO 96
95  WRITE(6,1038) MAXTRY

C
C      ----- END OF ITERATION LOOP -----
96  CONTINUE
      CALL MATINV(D,NP,P)

C
C      ----- WRITE CORRELATION MATRIX -----
      DO 98 I=1,NP
      E(I)=DSQRT(D(I,I))
98  IF (E(I).EQ.0.) E(I)=1.E-30
      IF (NP.EQ.1) GO TO 104
      WRITE(6,1013) (I,I=1,NP)
      DO 102 I=1,NP
      DO 100 J=1,I
100  A(J,I)=D(J,I)/(E(I)*E(J))
102  WRITE(6,1014) I,(A(J,I),J=1,I)
104  SUMC=0.0
      SUMF=0.0
      SUMC2=0.0
      SUMF2=0.0
      SUMCF=0.0
      DO 106 I=1,NOB
      SUMC=SUMC+C(I)
      SUMF=SUMF+F(I)
      SUMC2=SUMC2+C(I)*C(I)
      SUMF2=SUMF2+F(I)*F(I)
106  SUMCF=SUMCF+C(I)*F(I)
      RSQ=(SUMCF-SUMC*SUMF/NOB)**2/((SUMC2-SUMC*SUMC/NOB)*(SUMF2-SUMF*
1  SUMF/NOB))
      WRITE(6,1041) RSQ

```

```

C
C ----- CALCULATE 95% CONFIDENCE INTERVAL -----
Z=1./FLOAT(NOBS-NP)
SDEV=DSQRT(Z*SUMB)
TVAR=1.96+Z*(2.3779+Z*(2.7135+Z*(3.187936+2.466666*Z**2)))
WRITE(6,1015)
DO 108 I=1,NP
  SECOEF=E(I)*SDEV
  TVALUE=TH(I)/SECOEF
  TSEC=TVAR*SECOEF
  TMCOE=TH(I)-TSEC
  TPCOE=TH(I)+TSEC
  J=2*I-1
  IF(NP.EQ.1)
    1WRITE(6,1043) I,BI(J),BI(J+1),TH(I),SECOEF,TMCOE,TPCOE
  IF(NP.GT.1)
    1WRITE(6,1016) I,BI(J),BI(J+1),TH(I),SECOEF,TVALUE,TMCOE,TPCOE
108 CONTINUE
C
C ----- PREPARE FINAL OUTPUT -----
110 LSORT(1)=1
DO 116 J=2,NOB
  TEMP=R(J)
  K=J-1
DO 111 L=1,K
  LL=LSORT(L)
  IF(TEMP-R(LL)) 112,112,111
111 CONTINUE
  LSORT(J)=J
  GO TO 116
112 KK=J
113 KK=KK-1
  LSORT(KK+1)=LSORT(KK)
  IF(KK-L) 115,115,113
115 LSORT(L)=J
116 CONTINUE
  WRITE(6,1017)
DO 117 I=1,NOB
117 WRITE(6,1018) I,X(I),T(I),C(I),F(I),R(I)
  WRITE(6,1019)
DO 118 I=1,NOB
  J=LSORT(NOBS+1-I)
118 WRITE(6,1018) J,X(J),T(J),C(J),F(J),R(J)
  GO TO 150
120 WRITE(6,1028)
DO 125 I=1,NVAR
  J=2*I-1
  IF(I.EQ.4) GO TO 125
  IF((INDEX(I).EQ.1).AND.(B(I+6).NE.0.0)) WRITE(6,1029)BI(J),BI(J+1)
125 CONTINUE
  GO TO 150
129 WRITE(6,1032)
  GO TO 150
130 WRITE(6,1031)
  GO TO 150
140 WRITE(6,1030)
DO 145 I=1,NOB
145 WRITE(6,1033)I,X(I),T(I),F(I)
150 CONTINUE
C
C ----- END OF PROBLEM -----

```



```

999 FORMAT(2F10.4,15)
1000 FORMAT(1H1,10X,82(1H*)/11X,1H*,80X,1H*/11X,1H*,10X,'ONE-DIMENSIONAL
1L CONVECTION-DISPERSION EQ. SOLUTION',20X,1H*)
1001 FORMAT(20A4)
1002 FORMAT(11X,1H*,20A4,1H*)
1003 FORMAT(11X,1H*,80X,1H*/11X,82(1H*))
1004 FORMAT(7(4X,A4,A2))
1005 FORMAT(7F10.0)
1006 FORMAT(7I10)
1007 FORMAT(/11X,'INITIAL VALUES OF COEFFICIENTS'/11X,30(1H=)/12X,
1'NAME',11X,'INITIAL VALUE')
1008 FORMAT(11X,A4,A2,4(1H.),F12.4)
1009 FORMAT(/11X,'OBSERVED DATA',/11X,13(1H=)/11X,'OBS. NO.',5X,'CONCENTRATION',6X,'DISTANCE',9X,'TIME')
1010 FORMAT(11X,15,6X,F12.4,4X,F12.4,4X,F12.4)
1011 FORMAT(/11X,'ITERATION',6X,'SSQ',4X,5(7X,A4,A2))
1012 FORMAT(11X,15,3X,F13.5,2X,5(F13.5))
1013 FORMAT(/11X,'CORRELATION MATRIX'/11X,18(1H=)/14X,10(4X,12,5X))
1014 FORMAT(11X,13,10(2X,F7.4,2X))
1015 FORMAT(1H1,10X,'NON-LINEAR LEAST SQUARES ANALYSIS, FINAL RESULTS',
1/11X,48(1H=)//72X,'95% CONFIDENCE LIMITS'/11X,'VARIABLE',4X,'NAME',
2,8X,'VALUE',8X,'S.E. COEFF.',3X,'T-VALUE',5X,'LOWER',10X,'UPPER')
1016 FORMAT(14X,12,6X,A4,A2,1X,F13.5,3X,F13.5,2X,F8.2,1X,F13.5,
12X,F13.5)
1017 FORMAT(/11X,23(1H-),'ORDERED BY COMPUTER INPUT',24(1H-)/
150X,'CONCENTRATION',12X,'RESI-'/11X,'NO',4X,'DISTANCE',9X,'TIME',
210X,'OBS',9X,'FITTED',9X,'DUAL')
1018 FORMAT(10X,12,1X,F12.4,4(2X,F12.4))
1019 FORMAT(/11X,27(1H-),'ORDERED BY RESIDUAL',28(1H-)/
150X,'CONCENTRATION',12X,'RESI-'/11X,'NO',4X,'DISTANCE',9X,'TIME',
210X,'OBS',9X,'FITTED',9X,'DUAL')
1020 FORMAT(/11X,'END OF PROBLEM'/11X,14(1H=))
1021 FORMAT(11X,1H*,10X,'DETERMINISTIC LINEAR EQUILIBRIUM ADSORPTION FOR
1R PULSE INJECTION',7X,1H*/11X,1H*,10X,'WITH FIRST- AND ZERO-ORDER
2PRODUCTION AND DECAY',23X,1H*)
1022 FORMAT(11X,1H*,10X,'DETERMINISTIC TWO-SITE/TWO-REGION NONEQUILIBRIUM
MODEL FOR',8X,1H*/11X,1H*,10X,'PULSE-TYPE INJECTION WITH NO
2 PRODUCTION OR DECAY',22X,1H*)
1023 FORMAT(11X,1H*,10X,'STOCHASTIC TRANSPORT MODEL WITH EQUILIBRIUM ADSORPTION',
16X,1H*/11X,1H*,10X,'ZERO-ORDER PRODUCTION AND FIRST-ORDER DECAY FOR UNIFORM PULSE INPUT',
3X,1H*)
1024 FORMAT(11X,1H*,10X,'SOLUTION FOR RESIDENT CONCENTRATIONS',34X,1H*)
1025 FORMAT(11X,1H*,10X,'SOLUTION FOR FLUX CONCENTRATIONS',38X,1H*)
1026 FORMAT(11X,'CI',8(1H.),F12.4/11X,'CO',8(1H.),F12.4)
1028 FORMAT(/11X,'INPUT ERROR-- TOO MANY UNKNOWN!'/11X,'ONE OF THE FOLLOWING
COEFFICIENTS MUST BE FIXED:')
1029 FORMAT(60X,A4,A2)
1030 FORMAT(/11X,9(1H-),'RESULTS FOR INITIAL COEFFICIENT VALUES',9(1H-)/11X,
1NO',8X,'DISTANCE',8X,'TIME',7X,'CONCENTRATION')
1031 FORMAT(/11X,'INPUT ERROR: NONZERO INITIAL VALUES OF VARIABLE COEFFICIENTS
MUST BE SPECIFIED!'/11X,'FOR REACTION CONSTANTS BE SURE SIGN OF COEFFICIENT IS
CORRECT AS'/11X,'FITTING PROCEDURE WILL NOT CHANGE SIGNS')
1032 FORMAT(/11X,'INPUT ERROR: V,D AND R MUST BE GREATER THAN ZERO!')
1033 FORMAT(11X,12,3(3X,F12.4))
1034 FORMAT(/11X,'CONVERGENCE CRITERIA NOT MET IN',13,'ITERATIONS')
1035 FORMAT(11X,1H*,10X,'NON-LINEAR LEAST-SQUARES ANALYSIS',37X,1H*)
1036 FORMAT(11X,1H*,80X,1H*)
1037 FORMAT(11X,1H*,10X,'REDUCED CONCENTRATION DATA',44X,1H*)
1038 FORMAT(/11X,'NO FURTHER DECREASE IN SSQ OBTAINED AFTER',12,'TRIALS')
1039 FORMAT(11X,1H*,10X,'STOCHASTIC TRANSPORT MODEL WITH EQUILIBRIUM ADSORPTION',
16X,1H*/11X,1H*,10X,'ZERO-ORDER PRODUCTION AND FIRST-ORDER DECAY FOR UNIFORM
SOLUTE LOAD',3X,1H*)
1040 FORMAT(11X,1H*,10X,'REDUCED CONCENTRATION AND TIME DATA',35X,1H*)
1041 FORMAT(/11X,'RSQUARE FOR REGRESSION OF OBSERVED VS PREDICTED=',
1F10.8/)

```

```

1042 FORMAT(1H1,10X,'NON-LINEAR LEAST SQUARES ANALYSIS, FINAL RESULTS'
1/11X,48(1H=)//64X,'95% CONFIDENCE LIMITS'/11X,'VARIABLE',4X,'NAME'
2,8X,'VALUE',8X,'S.E. COEFF.',7X,'LOWER',10X,'UPPER')
1043 FORMAT(14X,12,6X,A4,A2,1X,F13.5,3X,F13.5,1X,F13.5,2X,F13.5)
STOP
END

C
SUBROUTINE MATINV(A,NP,B)
C
C PURPOSE: PERFORM MATRIX INVERSION FOR PARAMETER ESTIMATION
C
IMPLICIT REAL*8(A-H,O-Z)
DIMENSION A(7,7),B(14),INDEX(7,2)
DO 2 J=1,7
2 INDEX(J,1)=0
I=0
4 AMAX=-1.0
DO 10 J=1,NP
IF(INDEX(J,1)) 10,6,10
6 DO 10 K=1,NP
IF(INDEX(K,1)) 10,8,10
8 P=DABS(A(J,K))
IF(P.LE.AMAX) GO TO 10
IR=J
IC=K
AMAX=P
10 CONTINUE
IF(AMAX) 30,30,14
14 INDEX(IC,1)=IR
IF(IR.EQ.IC) GO TO 18
DO 16 L=1,NP
P=A(IR,L)
A(IR,L)=A(IC,L)
16 A(IC,L)=P
P=B(IR)
B(IR)=B(IC)
B(IC)=P
I=I+1
INDEX(I,2)=IC
18 P=1./A(IC,IC)
A(IC,IC)=1.0
DO 20 L=1,NP
20 A(IC,L)=A(IC,L)*P
B(IC)=B(IC)*P
DO 24 K=1,NP
IF(K.EQ.IC) GO TO 24
P=A(K,IC)
A(K,IC)=0.0
DO 22 L=1,NP
22 A(K,L)=A(K,L)-A(IC,L)*P
B(K)=B(K)-B(IC)*P
24 CONTINUE
GO TO 4
26 IC=INDEX(I,2)
IR=INDEX(IC,1)
DO 28 K=1,NP
P=A(K,IR)
A(K,IR)=A(K,IC)
28 A(K,IC)=P
I=I-1
30 IF(I) 26,32,26
32 RETURN
END

```

```

C      SUBROUTINE MODEL(BN,CXT)
C
C      PURPOSE: ASSIGN COEFFICIENTS AND ROUTE EXECUTION FOR MODELS
C
      IMPLICIT REAL*8(A-H,O-Z)
      DIMENSION BN(14),CXT(90)
      COMMON/MODAT1/X(90),T(90),INDEX(7),NOB,NREDU,NVAR
      COMMON/MODAT2/C1,C0,SOLOAD,XL,F1,F2,F3,F4,F5,R,VLNM,SDLN,DISP,XJ,
1TJ,MODE,MODT
      EXTERNAL CAVG
C
C      ----UPDATE COEFFICIENT ARRAY----
      K=0
      NU1=NVAR+1
      NU2=NVAR*2
      DO 2 I=NU1,NU2
        IF(INDEX(I-NVAR).EQ.0) GO TO 2
        K=K+1
        BN(I)=BN(K)
      2 CONTINUE
C
C      ----- ASSIGN PARAMETERS AND CHANGE FROM REDUCED TO ACTUAL TIMES
      V=BN(NU1)
      R=BN(NVAR+3)
      REDT=1.0
      IF(NREDU.NE.2) GO TO 8
      REDT=XL/V
      DO 4 I=1,NOB
      4 T(I)=T(I)*REDT
      8 IF(MODE.LE.6) F3=REDT*BN(NVAR+4)
      IF(MODE.GE.5) GO TO 150
      IF(MODE.GE.3) GO TO 100
      F1=V/R
      F2=BN(8)/R
      F4=BN(11)/R
      F5=BN(12)/R
C
C      ----SOLVE FOR MODELS 1 AND 2
      50 DO 75 J=1,NOB
        CALL MOD12(CXT(J),X(J),T(J))
      75 CONTINUE
      GO TO 350
C
C      ----SOLVE FOR MODELS 3 AND 4
      100 F1=V
      F2=V*XL/BN(8)
      F4=DMIN1(BN(11),0.9999D00)*R
      F5=BN(12)
      DO 125 J=1,NOB
        CALL MOD34(CXT(J),X(J),T(J))
      125 CONTINUE
      GO TO 350
C
C      ----SOLVE FOR MODELS 5 THRU 8
      150 F4=BN(12)/R
      F5=BN(13)/R
      SDLN=BN(14)
      VLNM=DLOG(V)-0.5*SDLN*SDLN
      DISP=BN(9)
      IF(MODE.GE.7) SOLOAD=BN(11)/C0

```

```

VMIN1=DEXP(VLNM-3.9*SDLN)
VMAX1=DEXP(VLNM+3.9*SDLN)
CMIN=DABS(CO-CI)*1.D-06
MODT=MODE
IF((MODT.EQ.5).OR.(MODT.EQ.7)) MODE=1
IF((MODT.EQ.6).OR.(MODT.EQ.8)) MODE=2
DO 175 J=1,NOB
XJ=X(J)
TJ=T(J)
PL=DMAX1(XJ/DISP,1.D-10)
VMIN=DMAX1(VMIN1,DMIN1(0.14*DLOG(PL)*XJ/TJ-.05,0.9*XJ/TJ)*R)
CALL LIMIT(VMIN,VMAX,VMAX1,CMIN)
CALL ROMB(CAVG,CXT(J),VMIN,VMAX)
IF(MODE.EQ.2) CXT(J)=CXT(J)/V
175 CONTINUE
MODE=MODT
350 CONTINUE
IF(NREDU.EQ.0) GO TO 360
DO 355 I=1,NOB
T(I)=T(I)/REDT
355 CXT(I)=(CXT(I)-CI)/(CO-CI)
360 CONTINUE
RETURN
END

C
SUBROUTINE MOD12(CXT,X,TIME)
C
C PURPOSE: TO CALCULATE C(X,T) FOR MODE=1
C
IMPLICIT REAL*8 (A-H,O-Z)
COMMON/MODAT2/CI,CO,DM1,XL,VR,DR,T0,RX1R,RXOR,R,DM2,DM3,DM4,DM5,
1DM6,MODE,MDMY
IF(RX1R.NE.0.0) GO TO 50

C
C -----CALCULATE C(X,T) FOR RX1R=0
C
DO 20 M=1,2
A=0.0
B=0.0
T=TIME+(1-M)*T0
IF(T.LE.0.0) GO TO 20
S2=(X-VR*T)/DSQRT(4.*DR*T)
S6=VR*X/DR
S7=(X+VR*T)/DSQRT(4.*DR*T)
E1=EXF(0.0D00,S2)
E2=EXF(S6,S7)
IF(MODE.EQ.2) GO TO 2
S3=VR*DSQRT(T/(DR*3.141593))
S4=(X-VR*T)*(VR*T-X)/(4*DR*T)
S5=-0.5*(1.+VR*X/DR+VR*VR*T/DR)
E3=EXF(S4,0.0D00)
GO TO 3
2 S3=0.0
S5=0.5
E3=0.0
3 A=0.5*E1+S3*E3+S5*E2
IF(M.EQ.2) GO TO 30
IF(RXOR.EQ.0.0) GO TO 10
IF(MODE.EQ.2) GO TO 5
S8=(X-VR*T+DR/VR)/(2*VR)
S9=DSQRT(T*0.079577/DR)*(X+VR*T+2.*DR/VR)

```

```

      S10=T/2.-DR/(2.*VR*VR)+(X+VR*T)**2./(4*DR)
      GO TO 7
5    S8=(X-VR*T)/(2.*VR)
      S9=0.0
      S10=-(X+VR*T)/(2.*VR)
7    B=RXOR*(T+S8*E1-S9*E3+S10*E2)
10   CXT=C1+(C0-C1)*A+B
20   CONTINUE
30   CX1=CXT-C0*A
      RETURN

```

C

C

```

      -----CALCULATE RX1R.NE.O. CASE
50  RAT=RXOR/RX1R
      DO 70 M=1,2
          A=0.0
          B=0.0
          T=TIME+(1-M)*T0
          IF(T.LE.0.0) GO TO 70
          S7=(X+VR*T)/DSQRT(4.*DR*T)
          IF(M.EQ.2) GO TO 55
          S1=-RX1R*T
          S2=(X-VR*T)/DSQRT(4.*DR*T)
          S6=VR*X/DR
          E1=EXF(0.0D00,S2)
          E2=EXF(S6,S7)
          IF(MODE.EQ.2) GO TO 52
          S3=VR*DSQRT(T/(DR*3.141593))
          S4=(X-VR*T)*(VR*T-X)/(4*DR*T)
          S5=-0.5*(1.+VR*X/DR+VR*VR*T/DR)
          E3=EXF(S4,0.0D00)
          GO TO 53
52  S3=0.0
          S5=0.5
          E3=0.0
53  A=EXF(S1,0.0D00)*(1.-0.5*E1-S3*E3-S5*E2)
55  CONTINUE
          U=DSQRT(VR*VR+4.*RX1R*DR)
          S9=(VR-U)*X/(2.*DR)
          S10=(X-U*T)/DSQRT(4.*DR*T)
          S12=(VR+U)*X/(2.*DR)
          S13=(X+U*T)/DSQRT(4.*DR*T)
          IF(MODE.EQ.2) GO TO 57
          S8=VR/(VR+U)
          S11=VR/(VR-U)
          S14=VR*VR/(2.*RX1R*DR)
          S15=VR*X/DR-RX1R*T
          S16=S14*EXF(S15,S7)
          GO TO 59
57  S8=0.5
          S11=0.5
          S16=0.0
59  B=S8*EXF(S9,S10)+S11*EXF(S12,S13)+S16
60  IF(M.EQ.2) GO TO 80
      CXT=(C1-RAT)*A+(C0-RAT)*B+RAT
70  CONTINUE
80  CXT=CXT-C0*B
      RETURN
      END

```

```

C
C SUBROUTINE MOD34(CXT,X,T)
C
C PURPOSE: CALCULATE C(X,T) FOR MODELS 3 AND 4
C
  IMPLICIT REAL*8 (A-H,O-Z)
  COMMON/MODAT2/C1,CO,DM1,XL,V,P,TO,BETR,OMEGA,R,DM2,DM3,DM4,Z,TT,
1 MODE,MDMY
  DIMENSION WG(5),XG(5)
  DATA XG/.04691008,.23076534,.50000000,.76923466,.95308992/
  DATA WG/.1184634,.2393143,.2844444,.2393143,.1184634/
  EXTERNAL CCO
  TRO=V*TO/XL
  TR=V*T/XL
  Z=X/XL
  IF(Z.GT.0.0) GO TO 5
  Z=1.D-10
  IF(MODE.EQ.3) GO TO 5
  CXT=0.0
  IF(T.LE.TO) CXT=CO
  RETURN
5 DO 20 M=1,2
  A=0.0
  TT=TR+(1-M)*TRO
  IF(TT.LE.0.) GO TO 20
  AP=DSQRT(1+.05*P*Z)
  TMAX=DMIN1(TT,BETR*(Z+40.*(1.+AP)/P))
  TMIN=DMAX1(0.000,BETR*(Z+40.*(1.-AP)/P))
  IF(TMAX.LE.TMIN) GO TO 15
  A1=0.0
  IF(TMIN.GT.0.) GO TO 10
  TMAX2=TMAX*1.D-4
  DO 8 I=1,5
  TAU=TMIN+(TMAX2-TMIN)*XG(I)
8 A1=A1+WG(I)*CCO(TAU)
  A1=A1*(TMAX2-TMIN)
  TMIN=TMAX2
10 CONTINUE
  CALL ROMB(CCO,A2,TMIN,TMAX)
  A=A1+A2
15 IF(M.EQ.2) GO TO 30
  CXT=C1+(CO-C1)*A
20 CONTINUE
30 CXT=CXT-CO*A
  RETURN
  END

```

```

C      SUBROUTINE LIMIT(VMIN,VMAX,VMAX1,CMIN)
C
C      PURPOSE:  TO CALCULATE INTEGRATION LIMITS FOR MODELS 5-8
C
C      IMPLICIT REAL*8 (A-H,O-Z)
C
C      ----- CALCULATE VMIN -----
      IF(VMIN.GE.VMAX1)RETURN
      F=(VMAX1/VMIN)**0.1
      DO 8 J=1,3
      DO 4 I=1,11
      CLST=CAVG(VMIN)
      IF(CLST.GT.CMIN/100.) GO TO 6
4     VMIN=VMIN*F
      VMAX=VMIN
      RETURN
6     SLOPE=(CAVG(VMIN*1.001)-CLST)/(0.001*VMIN)
      IF(SLOPE.GT.0.) GO TO 10
      VMIN=VMIN/F
8     F=F**0.1
      GO TO 20
10    IF(CLST.LT.CMIN) GO TO 20
      DO 12 I=1,5
      VMIN=VMIN-CLST/SLOPE
      IF(VMIN.LE.0.0)GO TO 15
      IF(CLST.LT.CMIN) GO TO 20
      CLST=CAVG(VMIN)
      SLOPE=(CAVG(VMIN*1.001)-CLST)/(0.001*VMIN)
      IF(SLOPE.LE.0.) GO TO 20
12    CONTINUE
      GO TO 20
15    VMIN=VMIN+CLST/SLOPE
C
C      ----- CALCULATE VMAX -----
20    F=(VMAX1/VMIN)**0.1
      VMAX=VMAX1
      DO 28 J=1,3
      DO 24 I=1,11
      CLST=CAVG(VMAX)
      IF(CLST.GT.CMIN/100.) GO TO 26
24    VMAX=VMAX/F
26    SLOPE=(CAVG(VMAX*1.01)-CLST)/(0.01*VMAX)
      IF(SLOPE.LT.0.) GO TO 30
      VMAX=VMAX*F
28    F=F**0.1
      RETURN
30    IF(CLST.LT.CMIN) RETURN
      DO 32 I=1,5
      VMAX=VMAX-CLST/SLOPE
      IF(CLST.LT.CMIN) RETURN
      CLST=CAVG(VMAX)
      SLOPE=(CAVG(VMAX*1.01)-CLST)/(0.01*VMAX)
      IF(SLOPE.EQ.0.) RETURN
32    CONTINUE
      RETURN
      END

```

```

C
C SUBROUTINE ROMB(FUNC, AREA, XMIN, XMAX)
C
C PURPOSE: PERFORM ROMBERG INTEGRATION ON LOG-TRANSFORMED INTERVAL
C
  IMPLICIT REAL*8(A-H,O-Z)
  DIMENSION R(9,9)
  AREA=0.0
  IF(XMIN.GE.XMAX)RETURN
  LEVEL=9
  STOPER=0.00005
  XLN1=DLOG(XMIN)
  XLN2=DLOG(XMAX)
  DX1=XLN2-XLN1
  SUM=0.5*XMIN*FUNC(XMIN)
  AREA=SUM*DX1
  R(1,1)=AREA
  DX2=DX1/2.
  INC=1
  DO 20 I=2, LEVEL
    XLN=XLN1+DX2
    DO 4 M=1, INC
      XE=DEXP(XLN)
      SUM=SUM+XE*FUNC(XE)
4    XLN=XLN+DX1
      AREA=SUM*DX2
      R(1,I)=AREA
      DO 17 J=2, I
        K=I+1-J
17    R(J,K)=(4.**(J-1)*R(J-1,K+1)-R(J-1,K))/(4.**(J-1)-1)
        IF(R(1,1).GT.0.0) GO TO 18
        IF(I.GT.4) RETURN
        GO TO 19
18    ERROR=DABS((R(1,1)-R(I-1,2))/R(1,1))
        IF((ERROR.LT.STOPER).AND.(I.GT.4)) RETURN
19    DX1=DX1/2.
        DX2=DX2/2.
20    INC=INC*2
    RETURN
  END

C
C FUNCTION CCO(TAU)
C
C PURPOSE: CALCULATE ARGUMENT IN INTEGRAL FOR MODELS 3 AND 4
C
  IMPLICIT REAL*8 (A-H,O-Z)
  COMMON/MODAT2/DM1,DM2,DM3,DM4,DM5,P,DM6,BETR,OMEGA,R,DM7,DM8,DM9,
1Z,T,MODE,MDMY
  CCO=0.0
  G1=EXF(P*(BETR*Z-TAU)*(TAU-BETR*Z)/(4.*BETR*TAU),0.000)
  IF(MODE.EQ.4) GO TO 5
  G2=DSQRT(P/(BETR*TAU))
  G=0.56419*G2*G1-P/(2.*BETR)*EXF(P*Z,G2/2.*(BETR*Z+TAU))
  GO TO 10
5  G=(Z/TAU)*DSQRT(P*BETR/(12.5664*TAU))*G1
10 IF(G.LT.1.D-07) RETURN
  A=OMEGA*TAU/BETR
  B=OMEGA*(T-TAU)/(R-BETR)
  CCO=G*GOLD(A,B)
  RETURN
  END

```



```

C      FUNCTION CAVG(V)
C
C      PURPOSE: CALCULATE ARGUMENT IN INTEGRAL FOR MODELS 5-8
C
      IMPLICIT REAL*8(A-H,O-Z)
      COMMON/MODAT2/CI,CO,SOLOAD,XL,VR,DR,TO,RX1R,RX0R,R,VLNM,SDLN,DISP,
1X,T,MODE,MODT
      VLN=DLOG(V)
      VR=V/R
      DR=VR*DISP
      IF(MODT.GE.7) TO=SOLOAD/V
      CALL MOD12(C,X,T)
      ARG=DMIN1(1.D02,(VLN-VLNM)*(VLN-VLNM)/(2.*SDLN*SDLN))
      PROB=DEXP(-ARG)/(2.50663*SDLN*V)
      IF(C.LT.1.D-70)C=0.D00
      IF(MODE.EQ.1) CAVG=PROB*C
      IF(MODE.EQ.2) CAVG=PROB*C*V
      RETURN
      END
C
C      FUNCTION EXF(A,B)
C
C      PURPOSE: TO CALCULATE EXP(A) ERFC(B)
C
      IMPLICIT REAL*8(A-H,O-Z)
      EXF=0.D00
      IF((DABS(A).GT.100.).AND.(B.LE.0.)) RETURN
      C=A-B*B
      IF((DABS(C).GT.100.).AND.(B.GE.0.)) RETURN
      IF(C.LT.-100.) GO TO 3
      X=DABS(B)
      IF(X.GT.3.0) GO TO 1
      T=1./(1.+3275911*X)
      Y=T*(.2548296-T*(.2844967-T*(1.421414-T*(1.453152-1.061405*T))))
      GO TO 2
1 Y=.5641896/(X+.5/(X+1./(X+1.5/(X+2./(X+2.5/X+1.)))))
2 EXF=Y*DEXP(C)
3 IF(B.LT.0.0) EXF=2.*DEXP(A)-EXF
      RETURN
      END

```

```

C      FUNCTION GOLD(X,Y)
C
C      PURPOSE: TO CALCULATE GOLDSTEIN'S J-FUNCTION J(X,Y)
C
      IMPLICIT REAL*8(A-H,O-Z)
      GOLD=0.0
      BF=0.0
      E=2.*DSQRT(DMAX1(0.00,X*Y))
      Z=X+Y-E
      IF(Z.GT.17.) GO TO 8
      IF(E.NE.0.) GO TO 2
      GOLD=DEXP(-X)
      RETURN
2     A=DMAX1(X,Y)
      B=DMIN1(X,Y)
      NT=11.+2.*B+0.3*A
      IF(NT.GT.25) GO TO 6
      I=0
      IF(X.LT.Y) I=1
      GXY=1.+I*(B-1.)
      GXY0=GXY
      GX=1.0
      GY=GXY
      GZ=1.0
      DO 4 K=1,NT
      GX=GX*A/K
      GY=GY*B/(K+1)
      GZ=GZ+GX
      GXY=GXY+GY*GZ
      IF((GXY-GXY0)/GXY.LT.1.D-08) GO TO 5
4     GXY0=GXY
5     GOLD=GXY*EXF(-X-Y,0.0000)
      GO TO 8
6     DA=DSQRT(A)
      DB=DSQRT(B)
      P=3.75/E
      BO=(.3989423+P*(.01328592+P*(.00225319-P*(.00157565-P*(.00916281-P
1*(.02057706-P*(.02635537-P*(.01647633-.00392377*P))))))/DSQRT(E)
      BF=BO*EXF(-Z,0.0000)
      P=1./((1.+3.275911*(DA-DB))
      ERF=P*(.2548296-P*(.2844967-P*(1.421414-P*(1.453152-P*1.061405))))
      P=0.25/E
      C0=1.-1.772454*(DA-DB)*ERF
      C1=0.5-Z*C0
      C2=0.75-Z*C1
      C3=1.875-Z*C2
      C4=6.5625-Z*C3
      SUM=.1994711*(A-B)*P*(C0+1.5*P*(C1+1.666667*P*(C2+1.75*P*(C3+P*(C4
1*(1.8-3.3*P*Z)+97.45313*P))))
      GOLD=0.5*BF+(.3535534*(DA+DB)*ERF+SUM)*BF/(BO*DSQRT(E))
8     IF(X.LT.Y) GOLD=1.+BF-GOLD
      RETURN
      END

```

## Virginia's Agricultural Experiment Stations

- 1—Blacksburg  
Virginia Tech
- 2—Steeles Tavern  
Shenandoah Valley Research Station
- 3—Orange  
Piedmont Research Station
- 4—Winchester  
Winchester Fruit Research Laboratory
- 5—Middleburg  
Virginia Forage Research Station
- 6—Warsaw  
Eastern Virginia Research Station
- 7—Suffolk  
Tidewater Research and Continuing Education Center
- 8—Blackstone  
Southern Piedmont Research and Continuing Education Center
- 9—Critz  
Reynolds Homestead Research Center
- 10—Glade Spring  
Southwest Virginia Research Station
- 11—Hampton  
Seafood Processing Research and Extension Unit

